KITSAP PUBLIC
HEALTH DISTRICT

345 6th Street, Suite 300
Bremerton, WA 98337
360-728-2235

HEALTH OFFICER DECISION

Application Type: Building Site Application - New

Memo #: 318627
Tax ID #: 4156-001-004-1006
RP ACCT ID: 2606333
Expiration: 04/30/2018

Property Information

11143 ROLLING BAY WALK NE
Bainbridge Island WA 98110

Contractor of Record

Contractor Name: MILLER BAY WATER COMPANY
Contractor Phone #: (360) 598-3505

Applicant

Margaret Dufresne
9335 22ND AVE NW
SEATTLE WA 98117

Waivers

Waiver Type	Memo #	Notes
Waiver Class A	318628	Reviewed & approved by JK
Waiver Class A	318629	Reviewed & approved by JK

Health Officer Decision for Onsite Sewage System

Approved (See Conditions Below)	Name of Inspector: *KERRIE Yanda*	Date: *10/23/2014*
Expiration date extended by 60 days to 12/31/2017		

Health Officer Decision for Water Supply

Approved (See Conditions Below)	Name of Inspector: *JOHN KIESS, R.S.*	Date: *10/07/2014*

Final Decision: Approved

KITSAP PUBLIC HEALTH DISTRICT

345 6TH STREET, SUITE 300
BREMERTON, WA 98337-1866
(360) 337-5235

Building Site Application (BSA) Residential For Onsite Sewage System and Water Supply

Official Use Only	Submittal Date: OCT 2 - 2014	Memo #: 318627
	Fee: #1610	SSI: (BSA)

A. BUILDING SITE INFORMATION

Building Site Address - Street, City, Zip Code:	Total Proposed Bedrooms:	Total Proposed Sewage Flow (Gallons):
11131 AND 11143 ROLLING BAY WALK N.E. BAINBRIDGE ISLAND 98110	4	480 GPD

Assessor Tax Account No.:	Lot No.:	Short Plat No.:	Property Size (SqFt):
4156-001-(004-0909) AND (005-0502)	—	—	19,166 SF

B. OWNER/APPLICANT INFORMATION

Name: ☒ Current Property Owner -OR- ☐ Applicant	Phone #:	E-Mail:
MARGARET A. DUFRESNE	(206) 491-3917	

Owner/Applicant Mailing Address - Street, City, State, Zip Code:
9335 22ND AVE NW SEATTLE, WASH. 98117-2716

C. APPLICATION TYPE SUMMARY (Check all fields that apply)

Use/System Type	Application Type:	Type of Structures:
☒ Single Family	☒ New	☒ Primary Residence
☐ Standard	☐ Re-Design	☐ Accessory Dwelling Unit
☐ Alternative	☐ Modification/Expansion	☐ Guest House
☐ Multi Family	☐ Repair	☒ Other: GARAGE/OUTBUILDING WITH PLUMBING
☐ Standard	☐ Repair/Replacement	☐ Non-Habitable Structures with Plumbing (describe below):
☐ Alternative	☒ Other (Describe Below)	
☒ Waiver(s) Proposed	MEMO 405827 RE DESIGN	

ACCEPTED
For Sewage and Water ONLY

D. WATER SUPPLY DETAIL (Attach Water Availability Letter if available)

		System Name:	System ID:
☐ Proposed	☒ Public	KITSAP CO. PUD No. 1 - NORTH BAINBRIDGE SYSTEM	599949
☒ Existing	☐ Private ☐ Individual ☐ 2 Party	Assessor Tax Account Numbers for Properties Served by Well	
		Water Connection 1 (Parcel with Well):	Water Connection 2 (Parcel connected to Well):

E. OWNER, APPLICANT OR AGENT AND DESIGNER ACKNOWLEDGEMENT

I certify that (1) the information contained in this application is true and accurate to the best of my knowledge; (2) the application represents my intended use of this property; and (3) any related building permits that I apply for will be consistent with the plans and specifications contained in this application.

I acknowledge and understand that I, along with my contractors, are responsible for adhering to the conditions of approval of this application, and are responsible for conforming to Kitsap County Board of Health regulations for onsite sewage systems (Ordinance 2008A-01) and water supply (Ordinance 1999-6).

I acknowledge and understand that the design, location, and construction of my onsite sewage system and/or well is/are critical and of a sensitive nature, and I agree to protect these areas required by the regulations.

I understand that once this application is submitted and/or approved, any changes to, or variations from, the information or conditions related to this plan may require a revised application submittal and/or could result in the revocation, denial, or suspension of this application or a related building permit and that this application will fully expire within 3 (three) years and 30 (thirty) days from the original date of application submittal.

I understand that I have the right to appeal the Health Officer's decision concerning this application pursuant to the regulations, and that approval of this application does not guarantee that a building permit will be issued.

MBH20 David Martin 10-1-2014
Signature: ☐ Owner ☐ Applicant ☒ Agent Date

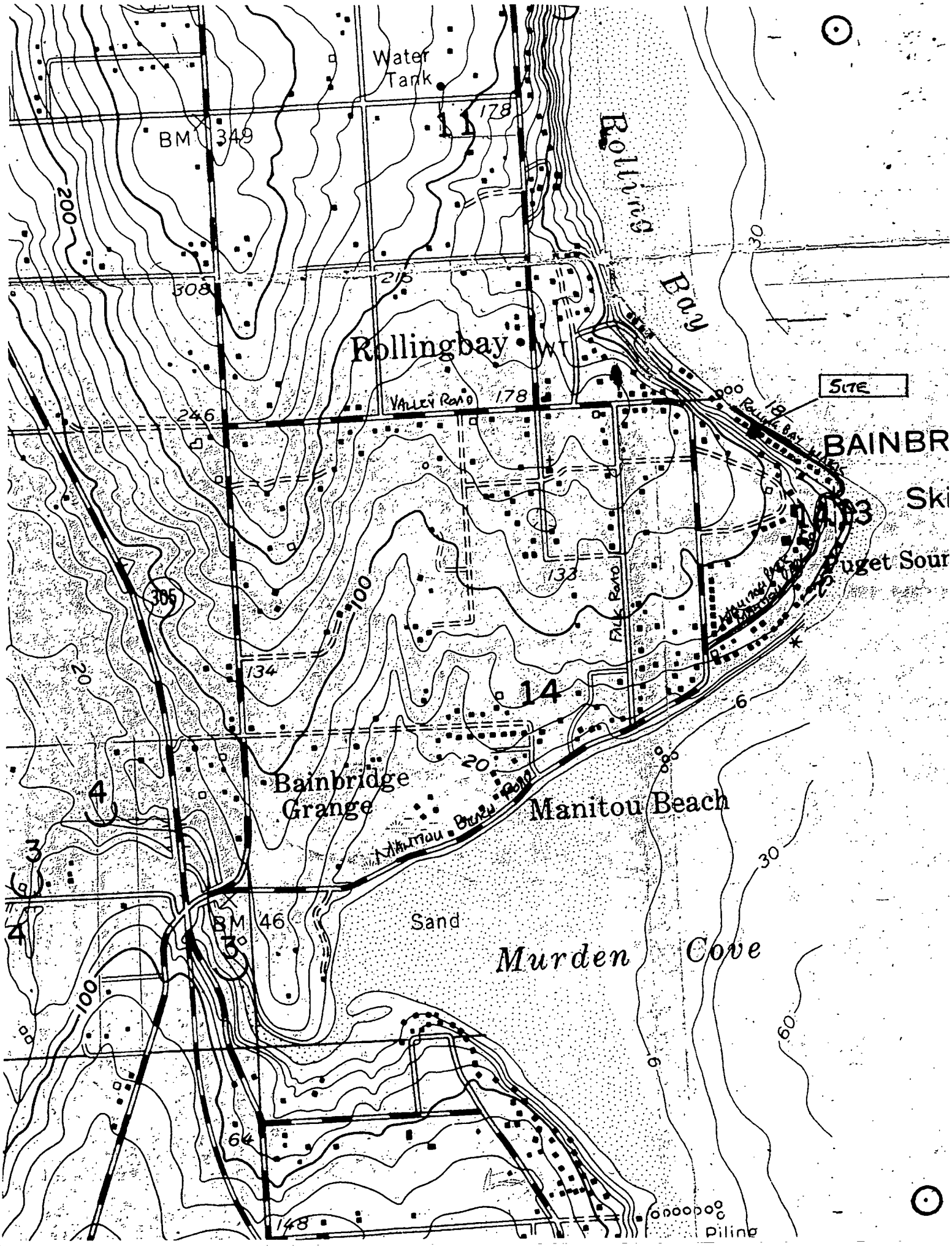
Designer/Engineer Stamp

ONSITE WASTEWATER TREATMENT SYSTEM
5100135
DAVID MARTIN
LICENSED DESIGNER
EXPIRES
10-1-14

(360) 598-3505
Designer/Engineer Contact Phone Number:
mbh20@earthlink.net
Designer/Engineer E-Mail Address:

F. RETURN CORRESPONDENCE (For Incomplete Applications Returned to Designer/Engineer)

Returned to Designer Date:	Application Re-submittal Date:


Water Tank
BM 349
Rolling Bay
Rollingbay
Valley Road
SITE
BAINBR
Ski
Puget Sour
Bainbridge Grange
Manitou Beach
Manitou Beach Road
Sand
Murden Cove
BM 46
Piling

KITSAP PUBLIC HEALTH DISTRICT

345 6TH STREET, SUITE 300
BREMERTON, WA 98337-1866
(360) 337-5235

Onsite Sewage System Specification Sheet For Residential Systems

Tax ID: 4156-001-(004-0009)(005-0502)

Owner/Applicant: DUFRESNE

G. SOIL EVALUATION PROFILES No Excavated Soil Logs

Soil Evaluation Date: Data from GeoEngineer and BSA's 409822 and 405824 2006

Soil Log Numbers Must Correlate With Site Plan - Indicate Total Excavated Depth, Soil Types, Water Table Level & Depth of Restrictive Layer

Soil Log #1 A	Soil Log #2 BSA 409822	Soil Log #3 BSA 409827	Soil Log #4
- Downslope Side Measurements -	- Downslope Side Measurements -	- Downslope Side Measurements -	- Downslope Side Measurements -
60" Colluvium/Slide Debris	60" Sand/Silt/Clay-Overburden	60" Sand/Silt/Clay Overburden	
210" Medium Dense Sand	132" Native Medium Sand	132" Native Medium Grain Sand	
216" Groundwater 6/2004			
		ACCEPTED For Sewage and Water ONLY	
Type 1/Type 2B A76014 1.0GPD/SF	Type 1/Type 2B @ 60" 1.0GPD/SF	Type 1/Type 2B 1.0GPD/SF	

H. DAILY FLOW - TANKAGE - TREATMENT PRE-APPLICATION PLAN

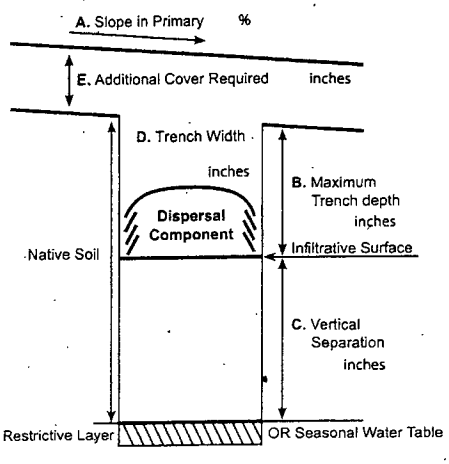
Design Flow	Tankage			Advanced Treatment
	Type	Size (gal)	QTY	
Total Proposed Sewage Flow/Day: 480 Gallons	☒ ATU Septic Tank	1000	1	☒ **Aerobic Treatment Unit**
Minimum Treatment Level	☒ Trash Tank	1000	1	☒ **Sand Filter** (includes bottomless)
Proposed Treatment Level: B+B	☒ Pump Tank	1250	1	☐ **Other:**
				Model/Size (Optional): ATU TO BSF
				Manufacturer (Optional):

I. DISPERSAL COMPONENT CONSTRUCTION

Dispersal Component Sizing	
Hydraulic Loading Rate of Dispersal Area:	1.0
Minimum Dispersal Area (Sq. Ft.) In Primary:	480 SF
Minimum Linear Feet or Dimensions:	10FT x 48FT

Distribution

- ☐ **Gravity Distribution**
- ☒ **Pressure Distribution**
- ☐ **Drip Irrigation**
- ☐ **Other:**



Trench Construction Profile

A. Percent Slope In Primary:	0	Precent
B. Maximum Trench Depth: - Downslope Side Measurements -	±72-96	inches
C. Vertical Separation:	>12	inches
D. Trench Width:	120	inches
E. Additional Cover Required:	0	inches

J. SITE WATER MITIGATION

☐ **Curtain Drain Designated** ☐ **Storm Water Control Designated**

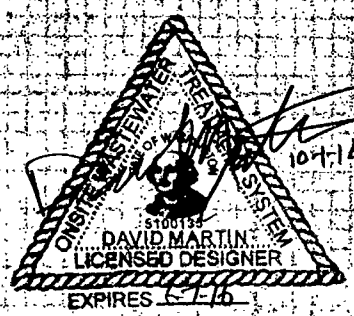
KITSAP COUNTY DEPARTMENT OF COMMUNITY DEVELOPMENT

BREMERTON-KITSAP COUNTY HEALTH DISTRICT

Site Plan Requirements Checklist

All site plans shall be clearly and accurately drawn to scale on paper no larger than 11" x 17" and must indicate all of the following information. For each item below, mark either "Shown" or "N/A" as appropriate for your project.
This checklist, completed and signed, must be included on all site plans. Any site plan without this checklist will be rejected and returned to the applicant for correction.

Shown	N/A	
		Parcel Number 4156-001-(004-0909)(005-0502)
A		**General Property Information:**
☒		Property lines and dimensions
☒		Elevations of property and the direction of natural drainage
☒		Slopes that exceed 15%, including any cut banks greater that 4" in height
☒		North arrow and site plan scale
☒	☐	Marine waters', lakes and ponds and their associated high water lines
☐	☒	Streams, creeks & wetlands and their associated buffer areas
B		**Existing Property Improvements:**
☐	☒	Location of all existing structures, including the locations of existing structures on adjacent waterfront properties
☐	☒	Location of all existing wells and their well radii, including those wells on adjacent properties within 100' of property lines
☐	☒	Location of all existing drainfields, including the 10' "No Build Zone" as well as the locations of existing drainfields on adjacent properties within 100' of any well
☐	☒	Location of existing drainage facilities, including all sub-surface infiltration systems
☒	☐	Location of all existing and abutting roadways, driveways, easements, buffers and required open spaces
C		**Proposed Property Improvements:**
☒	☐	Location and dimensions of all proposed structures or building envelopes in relation to property lines, other structures, etc.
☒	☐	Location of all proposed wells, including their 100' well radii and all water lines WATER LINE ONLY
☒	☐	Location of all proposed septic tanks, pump tanks, pre-treatment units, and drainfields, including the 10' "no build" zone
☐	☒	Location and dimensions of all proposed drainage and infiltration systems
☒	☐	Location, dimensions, surfacing materials, and clearing limits of all proposed parking areas, driveways, sidewalks and road appr's.
☒	☐	Location of all water, sewer and utility lines



DUPRESNE

KCDCD - ENVIRONMENTAL CHECKLIST
TAX LOT No. 4156-001-(004-0909)(005-0502)
PRE-APPLICATION PLAN

10-1-2014

KITSAP PUBLIC HEALTH DISTRICT

345 6th Street, Suite 300
Bremerton, WA 98337
360-728-2235

REVISION REQUEST FORM

Drinking Water & Onsite Sewage

Submittal Date	Memo Number	Review Fee
MAY 22 2018	063712	$65 mc

Please see the Environmental Health Fee Schedule for current fees.

BUILDING SITE ADDRESS

Street Address
11143 ROLLING BAY WALK N.E

City
BAINBRIDGE ISLAND 98110

Assessor's Account Number
4156-001-004-1006

OWNER OR APPLICANT INFORMATION

First Name *Last Name* *Contact Phone*
MARGARET DUFRESNE (206) 491-3917

Mailing Street Address
3866 N.E. ST HWY 104

Mail City *State* *Zip/Postal*
POULSBO, WASH. 98370

REVISION INFORMATION – THIS FORM IS TO BE USED ONLY IN ACCORDANCE WITH POLICY #3.

Original application memo #:
BSA MEMO No 318627

Describe minor revision in detail:
REDUCE FROM 2 DWELLING UNIT TO 1 4-BEDROOM DWELLING UNIT

– TANK RELOCATION REQUIRED – NO OTHER OSS REVISIONS REQUIRED

ENCLOSURES: SHEETS 1-2-3-4

SITE PLAN CHECKLIST APPROVED
FOR SEWAGE AND WATER ONLY

Reviewed + Approved 5-24-18 (SDB)

5-18-18
WASHINGTON ONSITE SEWAGE SYSTEM
5100135
DAVID MARTIN
LICENSED DESIGNER
EXPIRES 6-7-20

REVISED

Signature

Designer/Owner Name (Printed)
MBH2O DAVID MARTIN

Designer/Owner Name Signature
David Martin

Date
5-18-2018

KITSAP COUNTY DEPARTMENT OF COMMUNITY DEVELOPMENT

BREMERTON-KITSAP COUNTY HEALTH DISTRICT

Site Plan Requirements Checklist

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A		**General Property Information:**
■		Property lines and dimensions
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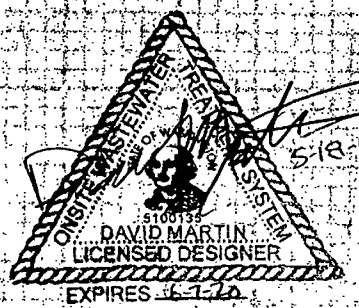
■	☐	Location of all water, sewer and utility lines



5-18-18

DUPRESNE

KCDCD - ENVIRONMENTAL CHECKLIST
TAX LOT No. 4156-001-004-1006

5-18-2018 MBH2O

NOTE:
- THIS IS NOT A PROPERTY SURVEY
- TRANSCRIBED TOPOGRAPHY (SHEET 2)

BUILDING SITE COMPLIANCE PLAN

1 - MIN. LAND AREA REQUIRED FOR UNIT 1 VALUE OF SEWAGE
- TYPE 2 SOILS - 12,500 SF MIN. LAND AREA
- LOT AGGREGATION OF TAX LOTS 004-09 AND 005-05 REQUIRED
- AGGREGATION NOTE & REFERENCE
- ADDRESS 11143 ROLLING BAY WALK
- TAX LOT NO 4156-001-004-0909

☐ PROPOSED STRUCTURE .:

1 - 4 BEDROOM HOME
- PRIMARY DWELLING UNIT (PDU)

⬡ PROPOSED OSS COMPONENTS

- SEE SHEETS 2, 3 & 4, AND BSA NOTES FOR DETAILS, AND SPECIFICATIONS
1 - 1000 GALLON SEPTIC TANK / TRASH TRAP
2 - MYCWATER BNR 500 ATU SYSTEM
3 - 1250 GALLON PUMP / DOSING TANK
4 - 480 GPD BOTTOMLESS INTERMITTENT SAND FILTER (BISF)
5 - 480 GPD BISF RESERVE AREA

BSA NOTES:

PAGE 1 - MYCWATER BNR 500 - SCHEMATIC
PAGE 2 - 1000 GALLON SEPTIC TANK / TRASH TRAP
PAGE 3 - 1250 GALLON PUMP / DOSING TANK
PAGE 4 - EFFLUENT PUMP CURVE
PAGE 5 - KPHD PRESSURE DISTRIBUTION WORKSHEET
PAGE 6 - SEWAGE EJECTOR PUMP SYSTEM - TYPICAL, IF REQUIRED
PAGE 7 - KPHD SAND FILTER SAND MEDIA STANDARD
PAGE 8. OPERATION AND MAINTENANCE (O&M)
- O&M TYPICAL SCHEDULE

PLAN SHEETS

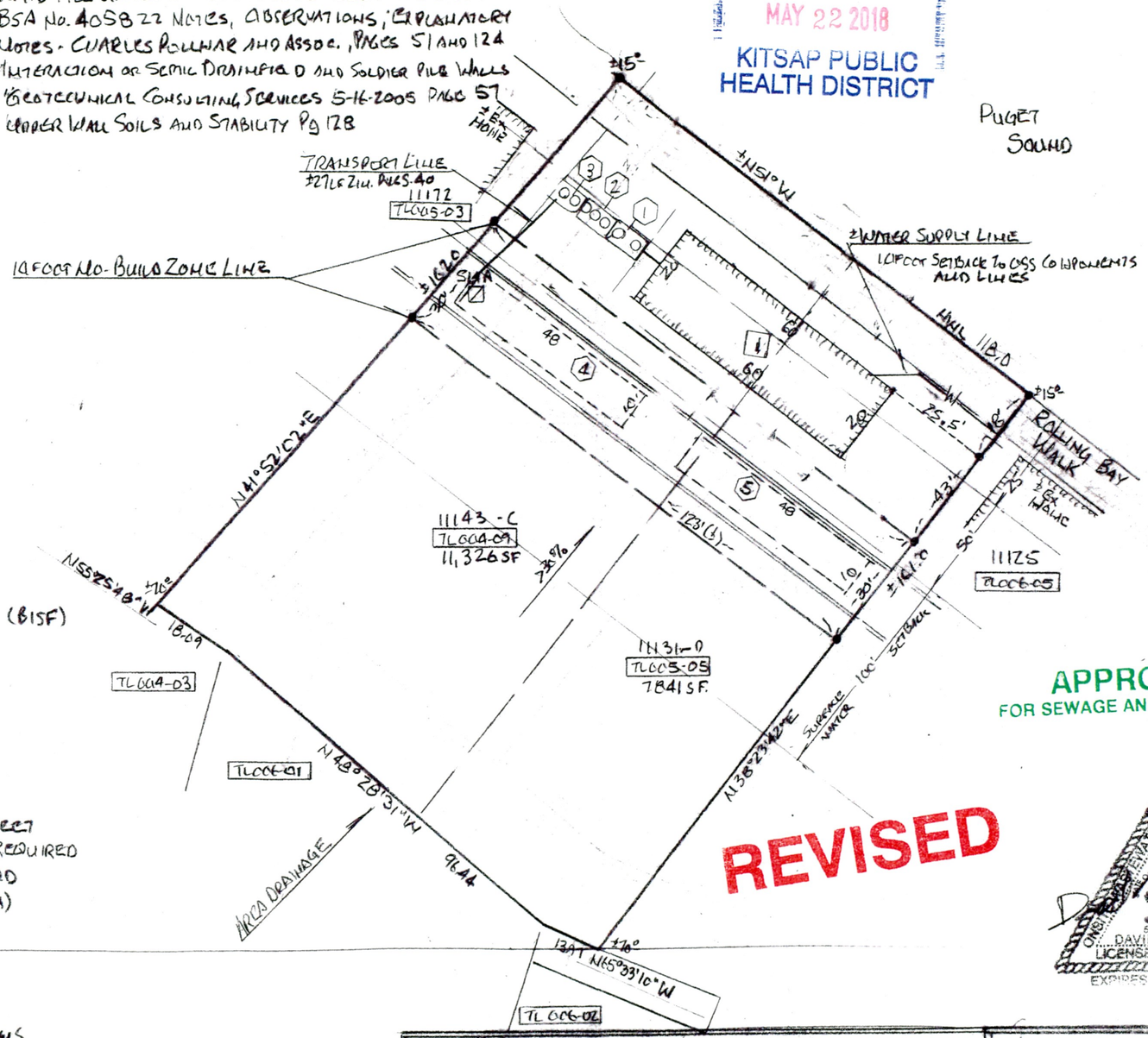
SHEET 1 - (THIS PAGE) - OSS SITE PLAN
SHEET 2 - OSS DETAIL SITE PLAN - SPECIFICATIONS
SHEET 3 - TREATMENT AND DISPERSAL SYSTEM CROSS-SECTION
SHEET 4 - BISF PROFILE - CONSTRUCTION STUDY NOTES

REFERENCES AND DOCUMENTS OF RECORD

1 - SOILS AND SLOPE STABILITY REFERENCES KPHD FILE UNDER PARCEL NO 4156-000-005-0502
2 - BSA No. 40S822 NOTES, OBSERVATIONS, EXPLANATORY NOTES - CHARLES POLLIAR AND ASSOC., PAGES 51 AND 124
3 - INTERACTION OF SEPTIC DRAINFIELD AND SOLDIER PILE WALLS "GEOTECHNICAL CONSULTING SERVICES 5-16-2005 PAGE 57 UPPER WALL SOILS AND STABILITY Pg 128

RECEIVED
MAY 22 2018
KITSAP PUBLIC HEALTH DISTRICT

SITE AREA - 0.26 ACRES + 0.18 AC. = 0.44 AC = 19,166 S.F.
AREA SOILS - DYSTRIC XEROTHERMENTS (45-70%) TO BEACHES
10-4



SCALE 1IN = 30 FEET
0 25 50 75 FEET

LEGEND:
- ☒ GEOTECHNICAL SOIL BORING 6/2008 (± LOCATION)
- ☐ PROPOSED STRUCTURE
- ⬡ PROPOSED OSS COMPONENTS
- KPHD COUNTY HEALTH DEPT.

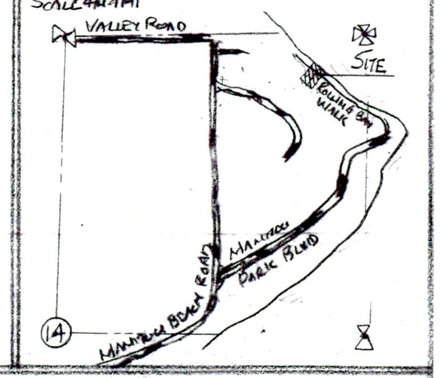
ON-SITE SEWAGE DISPOSAL SYSTEM (OSS) INSTALLATION NOTES:

1 - CONSTRUCTION ACCESS FROM BARGE-ON-BEACH FOR EXCAVATION EQUIPMENT AND OSS MATERIALS
2 - ALL WORK ON OSS SYSTEM BY KPHD AUTHORIZED INSTALLER ONLY - DRY SOIL CONDITIONS -
3 - INSTALL 480 GPD OSS SYSTEM PER SITE PLAN, DETAIL SHEETS, BSA NOTES, MANUFACTURER REQUIREMENTS, AND KPHD REGULATIONS
4 - PROPOSAL TO INSTALL PRIMARY AND RESERVE AREAS UNDER THIS PERMIT PLAN
5 - ALL EXCAVATED SOILS MUST BE REMOVED FROM SITE TO APPROVED DISPOSAL SITE

APPROVED
FOR SEWAGE AND WATER ONLY

REVISED

VICINITY MAP
SCALE 4in = 1MI



8-22-14	OSS - PRE-APPLICATION SITE PLAN	SHT 1 OF 2
10-1-14	OSS - SITE PLAN	SHT 1 OF 3
5-19-15	OSS - BUILDING SITE REVISION	SHT 1 OF 4

MARGARET DUFRESNE

ON-SITE SEWAGE DISPOSAL SYSTEM (OSS) - SITE PLAN
11131/11143 ROLLING BAY WALK N.E - BAINBRIDGE ISLAND
- PARCEL C BLA# 200503240117 / PARCEL D BLA# 200503240118
TAX LOT NO 4156-001-004-0909 / 4156-001-005-0502
SEC. 14 TWP 25N R2EW1M
SHEET 1 OF 4

AUG 11, 2014 MBH2O 118-24

NOTE: THIS IS NOT A PROPERTY SURVEY
TRANSCRIBED TOPOGRAPHY

☐ PROPOSED STRUCTURE

1 - 4-BEDROOM HOME
- PRIMARY DWELLING UNIT

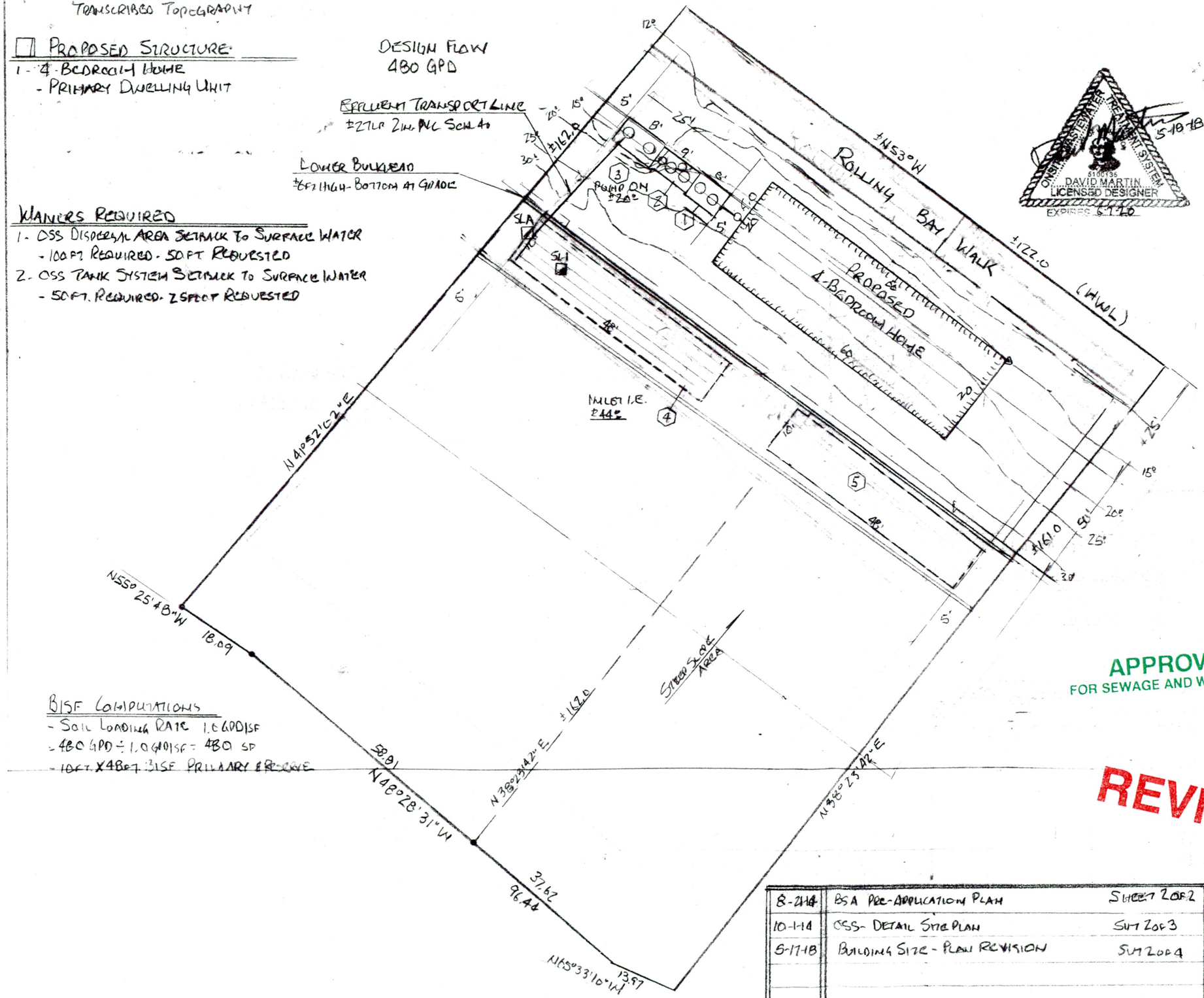
DESIGN FLOW
480 GPD

EFFLUENT TRANSPORT LINE
±277LF 2IN. PVC SCH 40

LOWER BULKHEAD
±6FT HIGH - BOTTOM AT GRADE

WAIVERS REQUIRED

1 - OSS DISPERSAL AREA SETBACK TO SURFACE WATER
- 100 FT REQUIRED - 50 FT REQUESTED
2 - OSS TANK SYSTEM SETBACK TO SURFACE WATER
- 50 FT. REQUIRED - 25 FT REQUESTED



5-10-18

5100135
DAVID MARTIN
LICENSED DESIGNER
EXPIRES 5-17-20

RECEIVED
MAY 22 2018
KITSAP PUBLIC HEALTH DISTRICT

SCALE 1 IN = 20 FEET
0 10 25 50 FEET

LEGEND:
- ☐ PROPOSED STRUCTURES
- ⬡ PROPOSED OSS COMPONENTS
- ☒ GEOTECH SOIL BORING (± LOCATION)
- ◎ SEWAGE PUMP LOCATIONS

⬡ PROPOSED OSS COMPONENTS

1 - 1000 GALLON SINGLE-COMPARTMENT, WATERTIGHT SEPTIC TANK/TRASH TRAP PRIMARY SETTLING TANK)
- INFLOW FROM SEWAGE PUMP SYSTEM IN BACH STRUCTURE

2 - 1000 GALLON NORWATER BNR 500 NITROGEN REDUCING AEROBIC TREATMENT UNIT (ATU), OR APPROVED EQUAL
3 - 1250 GALLON 2-COMPARTMENT WATERTIGHT PUMP/DOSING TANK WITH SCREENED/SURCODED EFFLUENT PUMP CAPABLE OF 20 GPM AT 32 FEET OF HEAD
- PUMP CONTROL BY PROGRAMMABLE TIMER SET FOR 240 GPD 20 GAL. PER DOSE 12 DOSES PER DAY
- PROVIDE MIN. 240 GALLON STORAGE ABOVE PUMP-ON

4 - 10FT. x 48FT. BOTTOMLESS INTERMITTENT SAND FILTER (BISF) - SEE SHEET 3
- 1 MIN. 6 MIN. INTO NATIVE SOILS - 8 FEET MAXIMUM DEPTH
- PLYWOOD BACKING 30 MIL PVC LINER TO 4FT. DEPTH
- SYSTEM CONSTRUCTION TO CONFORM TO KESDOH GUIDELINES FOR SAND FILTERS AND KPHD MEDIA STANDARDS - POLICY #24

5 - 480GPD BISF RESERVE AREA
- PROPOSE INSTALLATION AT TIME OF PRIMARY BISF SYSTEM INSTALLATION

APPROVED
FOR SEWAGE AND WATER ONLY

REVISED

BISF COMPUTATIONS

- SOIL LOADING RATE 1.0 GPD/SF
- 480 GPD ÷ 1.0 GPD/SF = 480 SF
- 10FT. X 48FT. BISF PRIMARY & RESERVE

8-21-18	BSA PRE-APPLICATION PLAN	SHEET 2 OF 2
10-1-14	OSS - DETAIL SITE PLAN	SHT 2 OF 3
5-17-18	BUILDING SIZE - PLAN REVISION	SHT 2 OF 4

DUFFRESNE

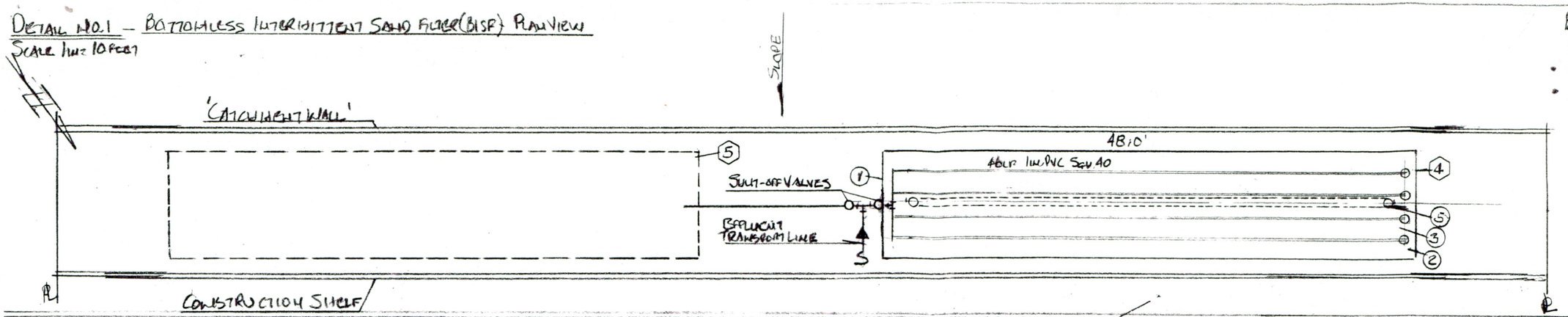
OSS - DETAIL SITE PLAN
T/L 4156 - 001 - (0014-0909) AND (005-0502)

SHEET 2 OF 4

AUG 14, 2014 MBH2O 11B-24

DETAIL NO.1 – BOTTOMLESS INTERMITTENT SAND FILTER (BISF) PLAN VIEW

SCALE 1in = 10 FEET



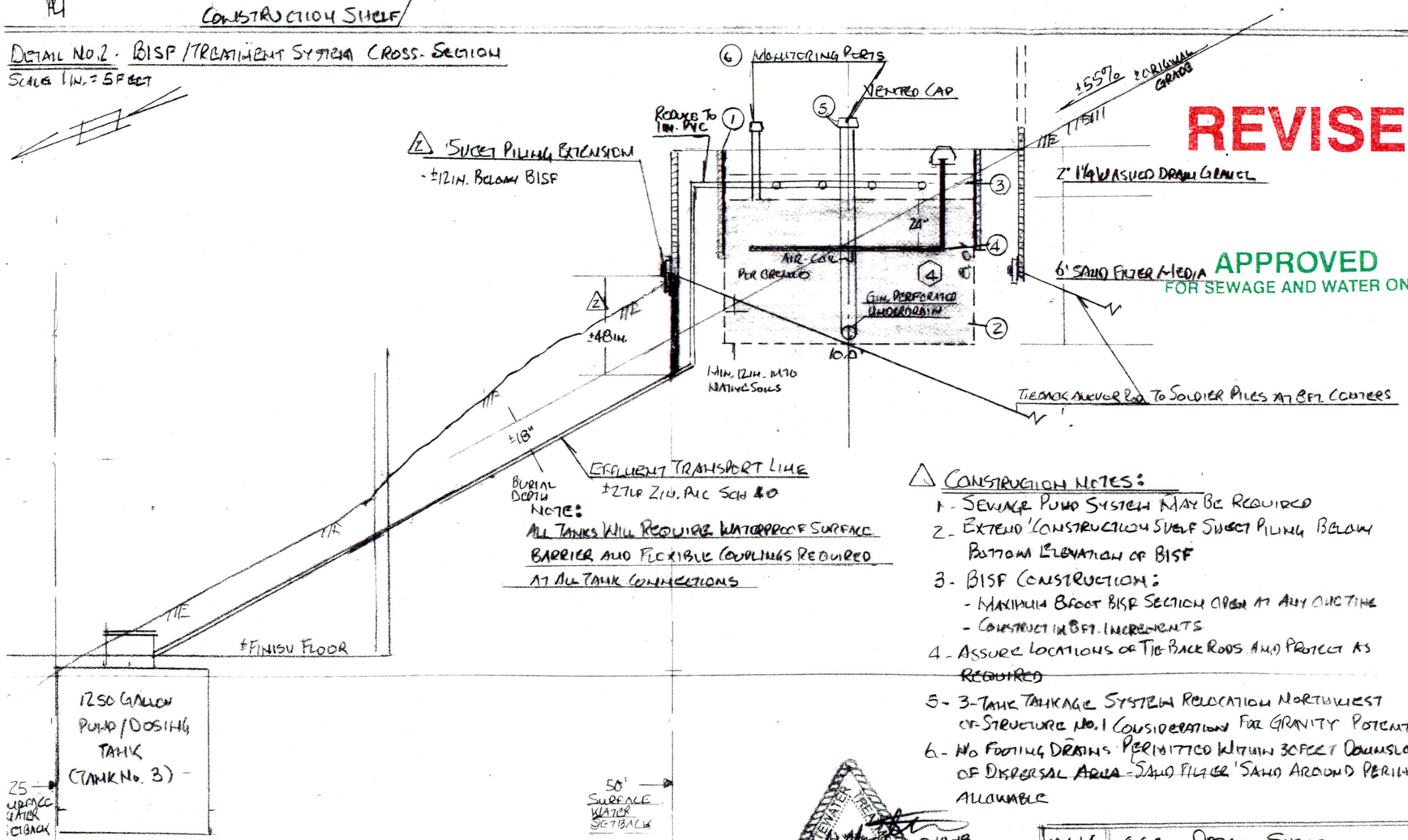
BISF SYSTEM DESIGN FLOW = 240 GPD

APPROXIMATE QUANTITIES

- 11 CY SAND FILTER SAND FOR BISF
- 4 CY 1¼" WASHED GRAVEL

DETAIL NO.2 – BISF / TREATMENT SYSTEM CROSS-SECTION

SCALE 1 IN. = 5 FEET



⬡ PROPOSED OSS COMPONENTS

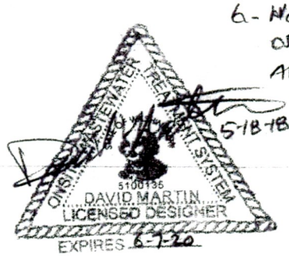
1 - 1000 GALLON SINGLE COMPARTMENT, WATERTIGHT SEPTIC TANK / TRASH TRAP
2 - NUWATER BNR 500 NITROGEN REDUCING AEROBIC TREATMENT UNIT (ATU)
3 - 1250 GALLON, 2-COMPARTMENT, WATERTIGHT PUMP/DOSING TANK
- INLET TO 1ST COMPARTMENT - PUMP CHAMBER TO 2ND COMPARTMENT
4 - 10 FT. x 48 FT. 240 GPD BOTTOMLESS INTERMITTENT SAND FILTER (BISF) TREATMENT AND DISPERSAL SYSTEM
6 - 480 GPD BISF RESERVE DRAINFIELD AREA
- RECOMMEND CONSTRUCTION OF RESERVE AREA CONCURRENT WITH PRIMARY BISF FOR FUTURE PERIODIC DOSING

○ BISF DETAILS & SPECIFICATIONS

- SYSTEM CONSTRUCTION TO COMPORT WITH WSDOH 'GUIDELINES FOR SAND FILTERS'
1 - 30 MIL PVC LINER WITH ±½in. PLYWOOD BACKING TO DEPTH OF 4 FEET
2 - SAND FILTER SAND PER KPHD POLICY #24 4-11-2011
3 - 1¼" WASHED DRAIN GRAVEL - FOR DISTRIBUTION BED
- EXTEND TO SURFACE TO MAXIMIZE AERATION POTENTIAL
4 - ±276 CF AIR-COIL, PER ORENCO, INC.
5 - 6in. PVC PERFORATED UNDERDRAIN PASSIVE VENTILATION SYSTEM
6 - MONITORING PORTS
A - 4in. PVC TO ABSORPTION BED / SAND MEDIA INTERFACE
B - 6in. UNDERDRAIN SAND MEDIA / NATIVE SOIL INTERFACE
7 - DISTRIBUTION SYSTEM (SEE BSA NOTES)
- 1in. PVC SCH 40 LATERALS WITH ⅛in. ORIFICES AT 48 IN. CENTERS
- 12:00 ORIENTATION WITH

△ CONSTRUCTION NOTES:

1 - SEWAGE PUMP SYSTEM MAY BE REQUIRED
2 - EXTEND 'CONSTRUCTION SHELF' SHEET PILING BELOW BOTTOM ELEVATION OF BISF
3 - BISF CONSTRUCTION:
- MAXIMUM 8 FOOT BISF SECTION OPEN AT ANY ONE TIME
- CONSTRUCT IN 8 FT. INCREMENTS
4 - ASSURE LOCATIONS OF TIE BACK RODS AND PROTECT AS REQUIRED
5 - 3-TANK TANKAGE SYSTEM RELOCATION NORTHWEST OF STRUCTURE No. 1 CONSIDERATION FOR GRAVITY POTENTIAL
6 - NO FOOTING DRAINS PERMITTED WITHIN 30 FEET DOWNSLOPE OF DISPERSAL AREA - 'SAND FILTER' SAND AROUND PERIMETER ALLOWABLE



5-18-18

10-1-14	OSS - DETAIL SHEET	SHT 3 OF 3
5-18-18	OSS - SITE PLAN REVISION	SHT 3 OF 4

DUPRESNE

OSS - DETAIL PLANS - TREATMENT DISPERSAL CROSS-SECTION
- BISF DETAILS & SPECIFICATIONS
TL 4156-001-004-0909 AND (005-0502) SHEET 3 OF 4

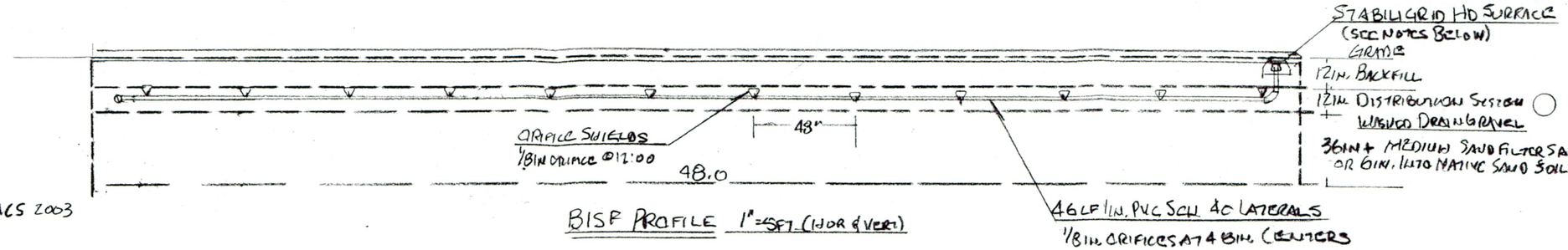
9-30-2014 MBH2O 118-24

Document Study

1. RBW Restoration LLC - CPCMAR 2006
2. Generalized Soil Conditions - Hydes Biodynamics 2003
3. Geo Engineer Letter to Shallwood 2005
4. Geotechnical Report - Aspect Consulting 2017
5. Geotechnical Revion - Mikel Foster Wheeler 2017

Document Study Notes:

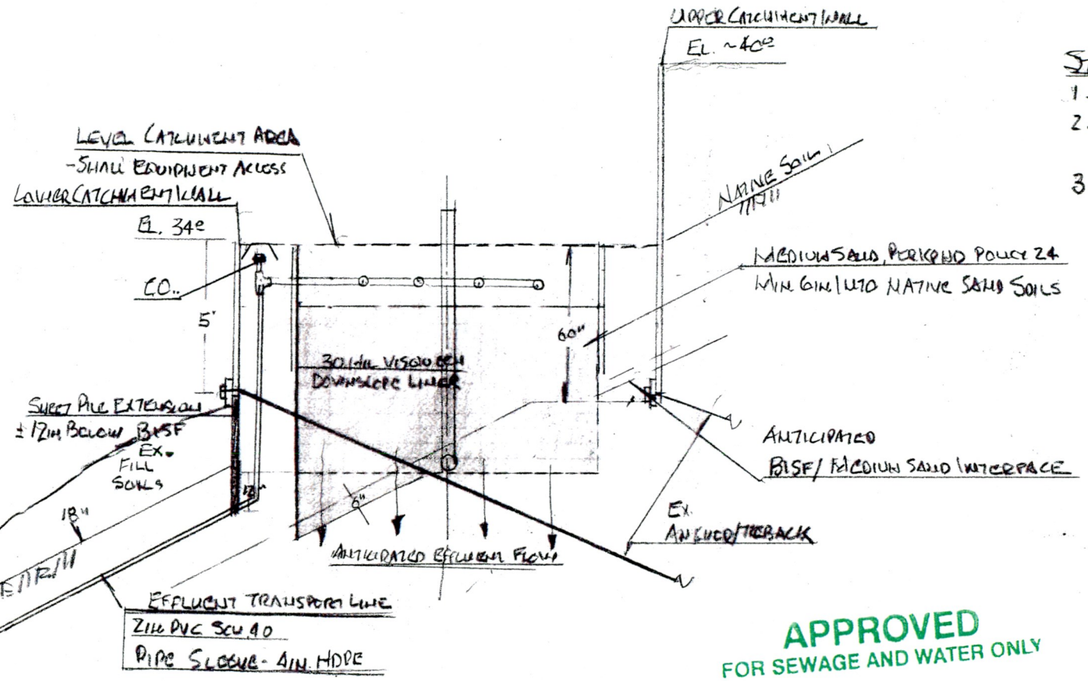
3. Timber Lagging Deterioration Potential
   - Effluent Seepage Expected To Infiltrate Vertically Into Native Sand Deposit
4. Timed Dosing of Small Increments:
   - 40 Gal. Dose 12 Times Daily
   - Transport Line Depth - 18in. Design 12in Min
   - Transport Line Sleeving - HDPE
5. Effluent Flow Through Retaining Wall
   - Vertical Flow Anticipated of Treated Wastewater
   - Equipment Access To Top of Catchment Area
   - Moderate Sized Equipment Access Maintain Into Location By Retaining Wall Engineers



BISF Profile 1"=5ft. (Hor & Vert)

12in Backfill
- Recommend Washed Drain Gravel To Surface

Stabilgrid Surface Grid System
1. Emplace Over Gravel Backfill
2. Minimize BISF Damage From Potential Debris or Tree Fall
3. Permit Maintenance Light Duty Equipment Operation



BISF - Cross-Section 1in=5ft

0 2 5 10 feet

RECEIVED
MAY 22 2018
KITSAP PUBLIC HEALTH DISTRICT

APPROVED
FOR SEWAGE AND WATER ONLY

REVISED

OSS - WATER TREATMENT SYSTEM
5100135
DAVID MARTIN
LICENSED DESIGNER
EXPIRES 6-7-18
5-18-18

5-18-18	BISF Dispersal System Detail	Sht 4 of 4	Dufresne
			OSS - Dispersal System Detail Plan T/L 4156-001 (004-0109) and (005-0502) Sheet 4 of 4
			Oct 19, 2017 MBH2O 118-24

Building Site Compliance Plan

1. Min. Land Area Required For Unit 1
   Village of Sewage
   - Type 2 Soils - 12,500 SF Min. Land Area
   - Lot Aggregation of Tax Lots 004-09 and 005-05 Required
   - Aggregation Note: Reference
   - Address 11143 Rolling Bay Walk
   - Tax Lot No. 4156-001-004-0909

☐ Proposed Structures:

1 - 3 Bedroom Home
   - Primary Dwelling Unit (PDU)
2 - 1 Bedroom Guest House - Garage Outbuilding with Plumbing
   - No Kitchen Facilities Permitted

⬡ Proposed OSS Components

- See Sheets 2 and 3, and BSA Notes for Details, Specifications

A. Sewage Pump Systems
1 - 1000 Gallon Septic Tank / Trash Trap
2 - Nuwater BNR 500 ATU System
3 - 1250 Gallon Pump / Dosing Tank
4 - 480 GPD Bottomless / Intermittent Sand Filter (BISF)
5 - 480 GPD BISF Reserve Area

BSA Notes:

Page 1 - Nuwater BNR 500 - Schematic
Page 2 - 1000 Gallon Septic Tank / Trash Trap
Page 3 - 1250 Gallon Pump / Dosing Tank
Page 4 - Effluent Pump Curve
Page 5 - KPHD Pressure Distribution Worksheet
Page 6 - Sewage Ejector Pump System - Typical
Page 7 - KPHD Sand Filter Sand / Media Standard
Page 8. Operation and Maintenance (O&M)
   - O&M Typical Schedule

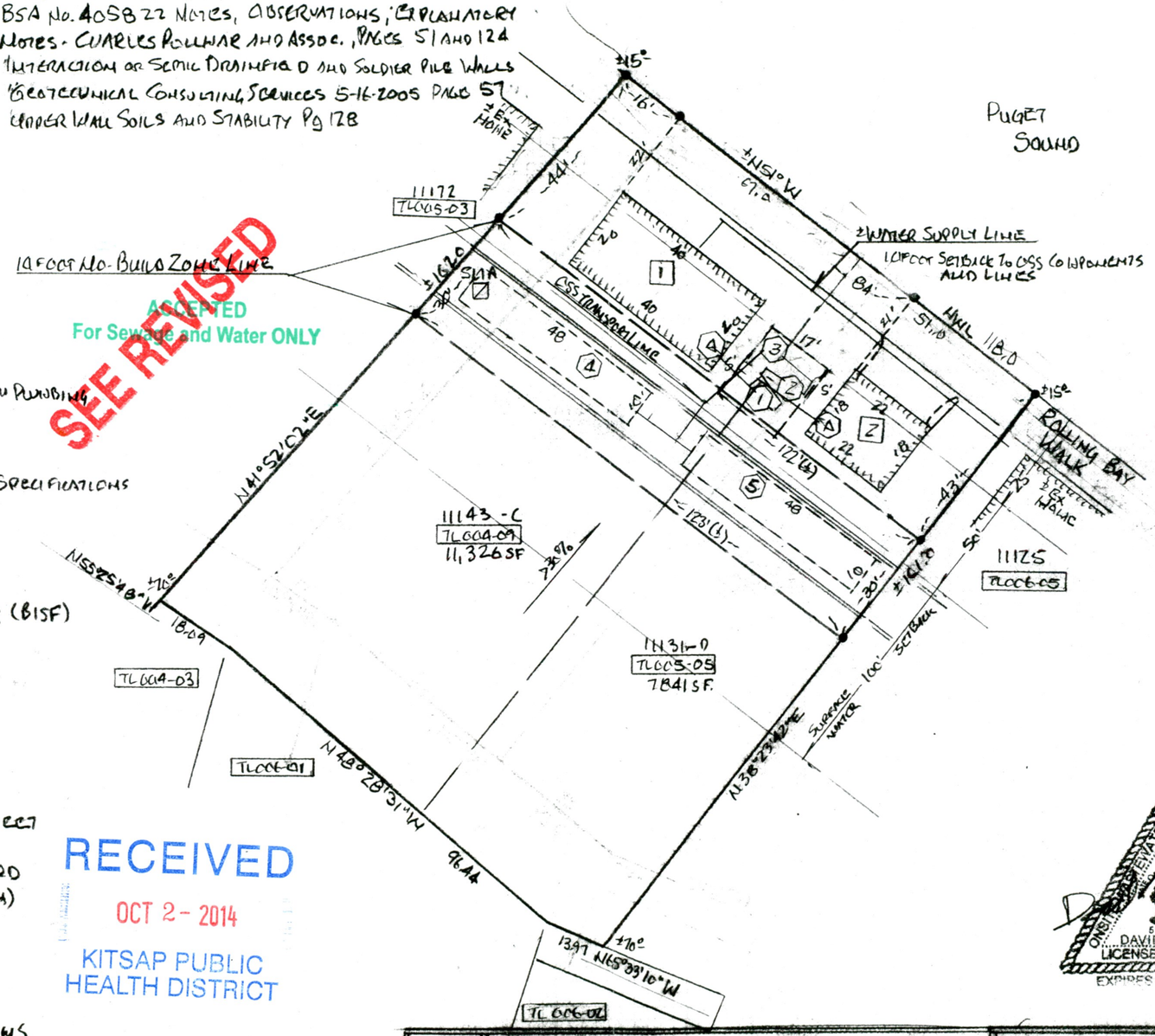
Plan Sheets

Sheet 1 - (This Page) - OSS Site Plan
Sheet 2 - OSS Detail Site Plan - Specifications
Sheet 3 - Treatment and Dispersal System Cross-Section (Detail No. 2)
   - BISF Detail Plan (Detail No. 1)

References and Documents of Record

1 - Soils and Slope Stability References
   KPHD File under Parcel No. 4156-001-005-0502
2 - BSA No. 40582 2 Notes, Observations, & Explanatory Notes - Charles Pollnac and Assoc., Pages 51 and 124
3 - Interaction of Septic Drainfield and Soldier Pile Walls "Geotechnical Consulting Services 5-16-2005 Page 57
   Upper Wall Soils and Stability Pg 128

Site Area - 0.26 Acres + 0.18 Ac. = 0.44 Ac = 19,166 S.F.
Area Soils - Dystric Xerorthents (45-70%) to Beaches
10-4



Scale 1 in = 30 feet
0 25 50 75 Feet

Legend:
☒ Geotechnical Soil Borings 6/2004 (± Location)
☐ Proposed Structures
⬡ Proposed OSS Components
KPHD County Health Dept.

On-Site Sewage Disposal System (OSS) Installation Notes:

1 - Construction Access From Barge-On-Beach For Excavation Equipment and OSS Materials
2 - All Work On OSS System By KPHD Authorized Installer Only - Dry Soil Conditions -
3 - Install 480 GPD OSS System Per Site Plan, Detail Sheets, BSA Notes, Manufacturer Requirements, and KPHD Regulations
4 - Proposal To Install Primary and Reserve Areas Under This Permit Plan
5 - All Excavated Soils Must Be Removed From Site To Approved Disposal Site

Vicinity Map
Scale 4 in = 1 Mi
Valley Road
Site
Rolling Bay Walk
Manitou Park Blvd
Manitou Beach Road
14

B-2-14	OSS - Pre-Application Site Plan	Sht 1 of 2
10-1-14	OSS - Site Plan	Sht 1 of 3

Margaret Dufresne

On-Site Sewage Disposal System (OSS) - Site Plan
11131 / 11143 Rolling Bay Walk N.E. - Bainbridge Island
- Parcel C BLA# 200503240117 / Parcel D BLA# 200503240118
Tax Lot No. 4156-001-004-0909 / 4156-001-005-0502
Sec. 14 Twp 25N R2EWM

Sheet 1 of 3

Aug 11, 2014 — MBH2O — 11B-24

☐ PROPOSED STRUCTURES

1 - 3-BEDROOM HOME
- PRIMARY DWELLING UNIT (PDU)

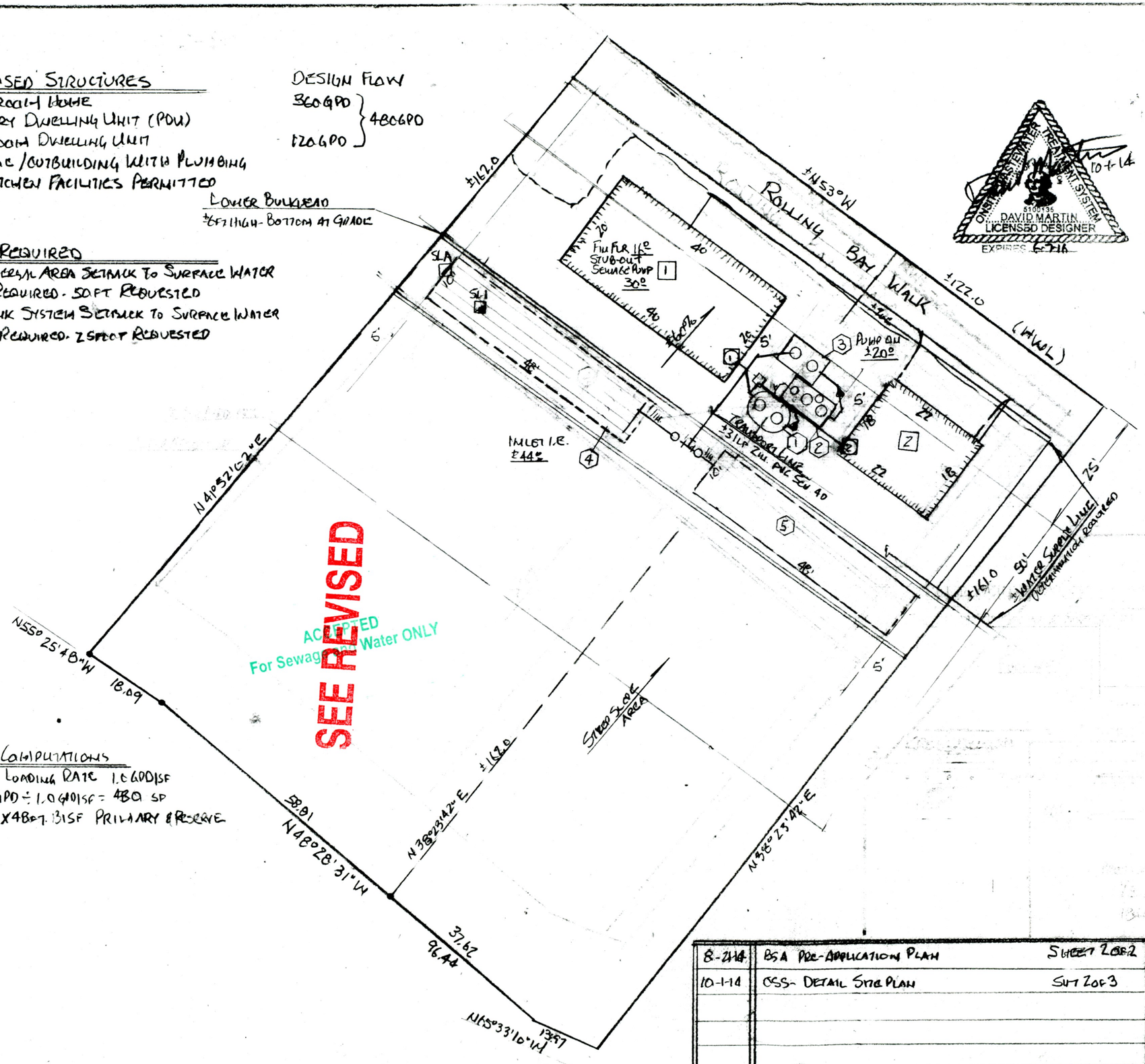
2 - 1 BEDROOM DWELLING UNIT
- GARAGE/OUTBUILDING WITH PLUMBING
- NO KITCHEN FACILITIES PERMITTED

DESIGN FLOW
360 GPD
120 GPD
} 480 GPD

LOWER BULKHEAD
±6FT HIGH - BOTTOM AT GRADE

WAIVERS REQUIRED

1. OSS DISPERSAL AREA SETBACK TO SURFACE WATER
   - 100FT REQUIRED - 50FT REQUESTED
2. OSS TANK SYSTEM SETBACK TO SURFACE WATER
   - 50FT. REQUIRED - 25FEET REQUESTED



BISF COMPUTATIONS

- SOIL LOADING RATE 1.0 GPD/SF
- 480 GPD ÷ 1.0 GPD/SF = 480 SF
- 10FT. X 48FT. BISF PRIMARY & RESERVE

SCALE 1IN:20 FEET
0 10 20 40 50 FEET

LEGEND:
- ☐ PROPOSED STRUCTURES
- ⬡ PROPOSED OSS COMPONENTS
- ☒ GEOTECH SOIL BORING (± LOCATION)
- ◎ SEWAGE PUMP LOCATIONS

⬡ PROPOSED OSS COMPONENTS

1 - 1000 GALLON SINGLE-COMPARTMENT, WATERTIGHT SEPTIC TANK/TRASH TRAP PRIMARY SETTLING TANK)
- INFLOW FROM SEWAGE PUMP SYSTEM IN EACH STRUCTURE

2 - 1000 GALLON NORWATER BNR 500 NITROGEN REDUCING AEROBIC TREATMENT UNIT (ATU), OR APPROVED EQUAL

3 - 1250 GALLON 2-COMPARTMENT WATERTIGHT PUMP/DOSING TANK WITH SCREENED/SHROUDED EFFLUENT PUMP CAPABLE OF 20 GPM AT 32 FEET OF HEAD
- PUMP CONTROL BY PROGRAMMABLE TIMER SET FOR 240 GPD 20 GAL. PER DOSE 12 DOSES PER DAY
- PROVIDE MIN. 240 GALLON STORAGE ABOVE PUMP-ON

4 - 10FT. X 48FT. BOTTOMLESS INTERMITTENT SAND FILTER (BISF) - SEE SHEET 3
- 1 MIN. 6IN. INTO NATIVE SOILS - 8 FEET MAXIMUM DEPTH
- PLYWOOD BACKING 30 MIL PVC LINER TO 4FT. DEPTH
- SYSTEM CONSTRUCTION TO CONFORM TO KCSDOH GUIDELINES FOR SAND FILTERS AND KPUD MEDIA STANDARDS - POLICY #24

5 - 480 GPD BISF RESERVE AREA
- PROPOSE INSTALLATION AT TIME OF PRIMARY BISF SYSTEM INSTALLATION

8-2-14	BSA PRE-APPLICATION PLAN	SHEET 2 OF 2
10-1-14	OSS - DETAIL SITE PLAN	SHT 2 OF 3

DUFFRESNE

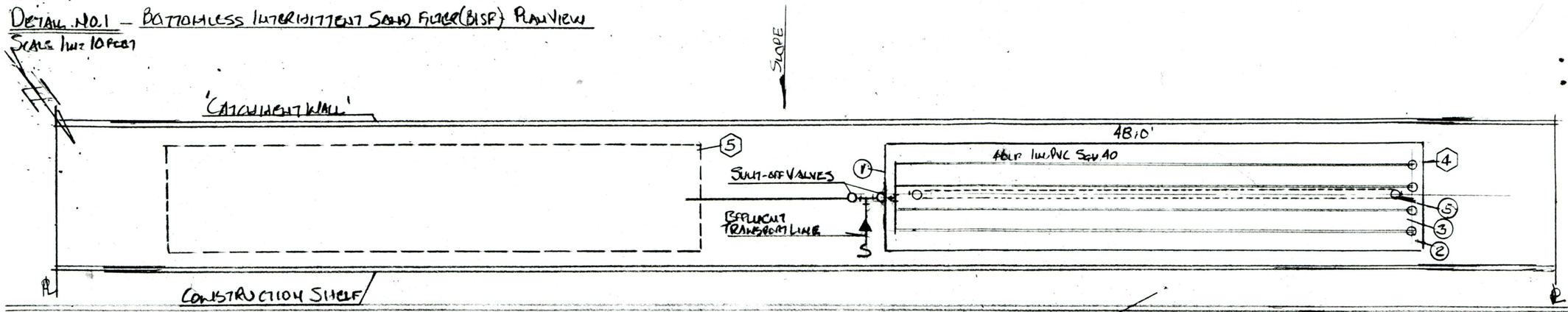
OSS - DETAIL SITE PLAN
T/L 4156 - 001 - (0014-0019) AND (005-0502)

SHEET 2 OF 3

AUG 14, 2014 MBH2O 118-24

DETAIL NO.1 – BOTTOMLESS INTERMITTENT SAND FILTER (BISF) PLAN VIEW

SCALE 1in = 10 FEET



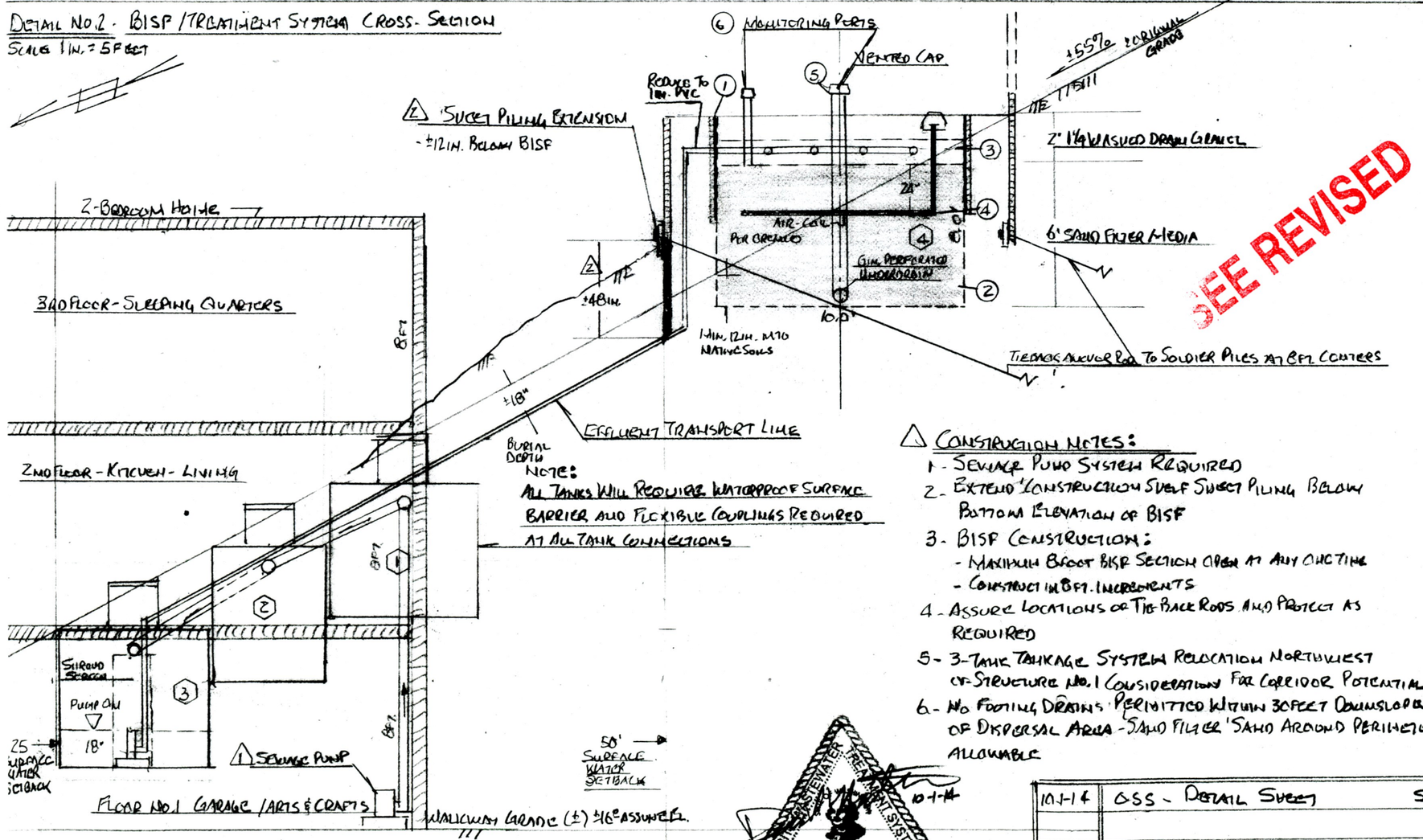
BISF SYSTEM DESIGN FLOW = 240 GPD

APPROXIMATE QUANTITIES

- 81 CY SAND FILTER SAND PER BISF
- 40 CY 1¼" WASHED GRAVEL

DETAIL NO.2 - BISF / TREATMENT SYSTEM CROSS-SECTION

SCALE 1 IN. = 5 FEET



CONSTRUCTION NOTES:

1. SEWAGE PUMP SYSTEM REQUIRED
2. EXTEND 'CONSTRUCTION SHELF' SHEET PILING BELOW BOTTOM ELEVATION OF BISF
3. BISF CONSTRUCTION:
   - MAXIMUM 8 FOOT BISF SECTION OPEN AT ANY ONE TIME
   - CONSTRUCT IN 8 FT. INCREMENTS
4. ASSURE LOCATIONS OF TIE BACK RODS AND PROTECT AS REQUIRED
5. 3-TANK TANKAGE SYSTEM RELOCATION NORTHWEST OF STRUCTURE NO.1 CONSIDERATION FOR CORRIDOR POTENTIAL
6. NO FOOTING DRAINS PERMITTED WITHIN 30 FEET DOWNSLOPE OF DISPERSAL AREA - SAND FILTER SAND AROUND PERIMETER ALLOWABLE

PROPOSED OSS COMPONENTS

1. 1000 GALLON SINGLE COMPARTMENT, WATERTIGHT SEPTIC TANK / TRASH TRAP
2. NORWATER BNR 500 NITROGEN REDUCING AEROBIC TREATMENT UNIT (ATU)
3. 1250 GALLON, 2-COMPARTMENT, WATERTIGHT PUMP/DOSING TANK
   - INLET TO 1ST COMPARTMENT - PUMP CHAMBER TO 2ND COMPARTMENT
4. 10 FT. x 48 FT. 240 GPD BOTTOMLESS INTERMITTENT SAND FILTER (BISF) TREATMENT AND DISPERSAL SYSTEM
6. 480 GPD BISF RESERVE DRAINFIELD AREA
   - RECOMMEND CONSTRUCTION OF RESERVE AREA CONCURRENT WITH PRIMARY BISF FOR FUTURE PERIODIC DOSING

BISF DETAILS & SPECIFICATIONS

- SYSTEM CONSTRUCTION TO COMPORT WITH WSDOH 'GUIDELINES FOR SAND FILTERS'
1. 30 MIL PVC LINER WITH ±½IN. PLYWOOD BACKING TO DEPTH OF 4 FEET
2. SAND FILTER SAND PER KPHD POLICY #24 4-11-2011
3. 1¼" WASHED DRAIN GRAVEL - FOR DISTRIBUTION BED
   - EXTEND TO SURFACE TO MAXIMIZE ABSORPTION POTENTIAL
4. ±276 LF AIR-COIL, PER ORENCO, INC.
5. 6in. PVC PERFORATED UNDERDRAIN PASSIVE VENTILATION SYSTEM
6. MONITORING PORTS
   A. 4in. PVC TO ABSORPTION BED / SAND MEDIA INTERFACE
   B. 6in. UNDERDRAIN SAND MEDIA / NATIVE SOIL INTERFACE
7. DISTRIBUTION SYSTEM (SEE BSA NOTES)
   - 1in. PVC SCH 40 LATERALS WITH ⅛in. ORIFICES AT 48 IN. CENTERS
   - 12:00 ORIENTATION WITH

5100136
DAVID MARTIN
LICENSED DESIGNER
EXPIRES 10-1-16
10-1-14

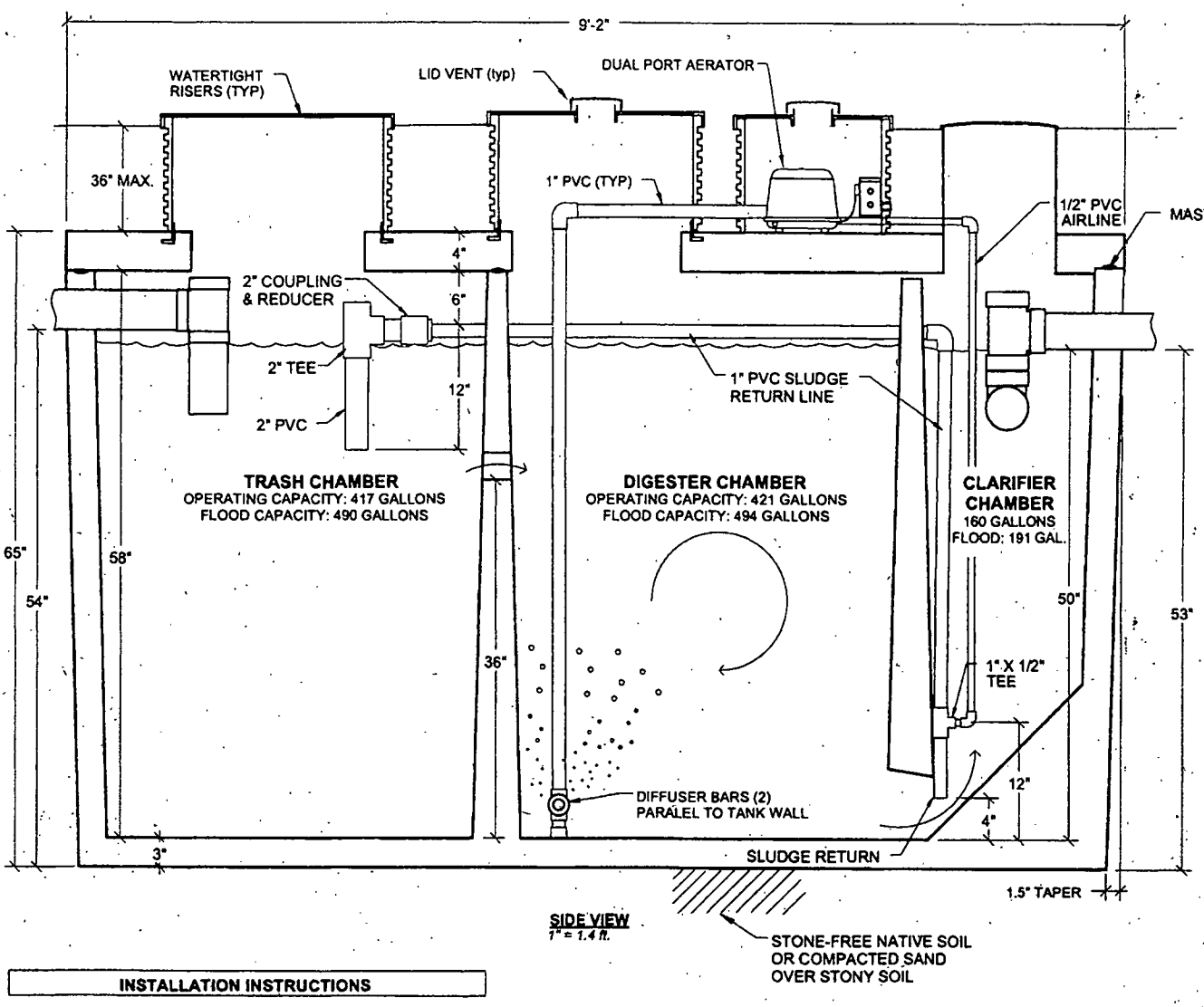
10-1-14	OSS - DETAIL SHEET	SHT 3 OF 3

DUFRESNE

OSS - DETAIL PLANS - TREATMENT DISPERSAL CROSS-SECTION
- BISF DETAILS & SPECIFICATIONS

TL #156-001-004-0909 AND (005-0502) SHEET 3 OF 3

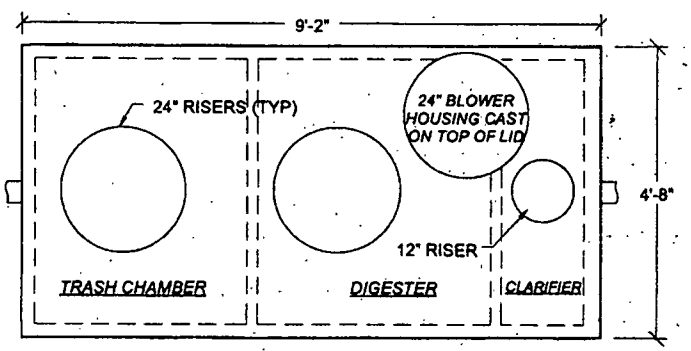
9-30-2014 MBH2O 118-24



SIDE VIEW
1" = 1.4 ft.

INSTALLATION INSTRUCTIONS

1) Excavate tank hole with vertical walls to 1 foot larger than tank on all sides.
2) If bottom of hole is stony, install 3" of compact sand & level out with screed.
3) Install tank in center of hole, keeping 1 ft. void space on all sides.
4) As tank is filling with water, fill in void space with compact granular (sandy) soil free of large clumps of clay.
5) Install rest of system, & affix risers to adapters with waterproof adhesive.
6) Perform watertightness test in field as required by local jurisdiction.
7) Upon approval to backfill, carefully backfill with native soils over top of tank.
8) Final grade the surface to avoid chanelling surface water toward tank.



TOP VIEW
1" = 2.8 ft.

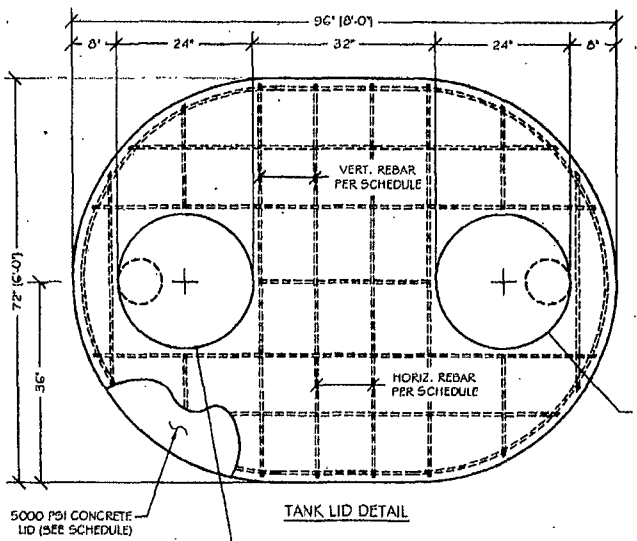
AEROBIC TREATMENT TANK DETAIL FOR NuWATER BNR-500 TREATMENT UNIT

ENVIRO-FLO, INC.
Wastewater Treatment Technologies
P.O. BOX 321161, Flowood, MS 39232
(877) 836-8476 (601) 845-4716 fax
www.enviro-flo.net

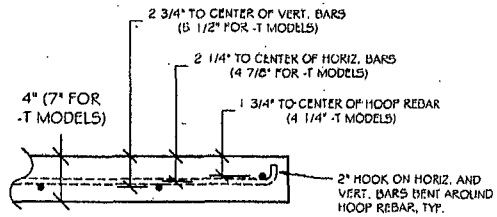
REVISED: 3/01/12

SCALE: 1" = 1.4 ft.

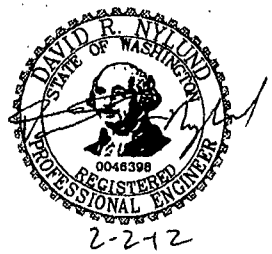
HAGERMAN PRE-CAST
Bob & Anita Hagerman
PO Box 2842
Poulsbo WA 98370
360-598-6121 / 360-509-3702



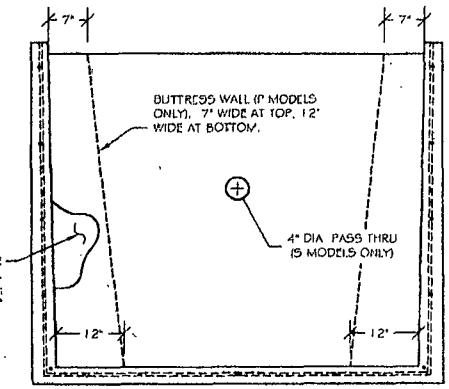
TANK LID DETAIL



DETAILS FOR LID REINFORCEMENT



2-2-12

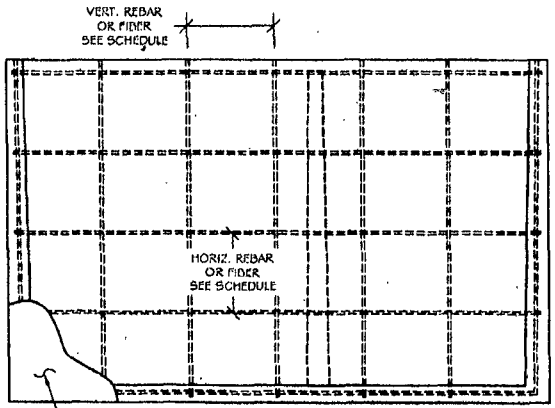


INTERCOMPARTMENTAL WALL DETAIL (BUTTRESS WALL FOR "P" MODELS)

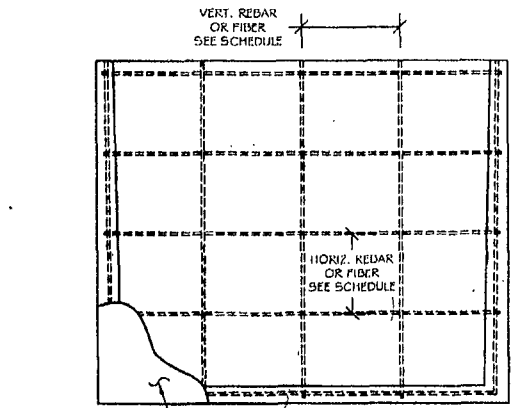
REINFORCEMENT SCHEDULE FOR 1000 S & 1000 P (3' OF SOIL COVER; WATER TABLE AT TOP OF TANK; 2,500 lb WHEEL LOAD)			
SIDE WALLS, END WALLS, INTERCOMPARTMENTAL WALLS, AND FLOOR		LID (THICKNESS 4")	
		DIAMETER	#4 (1/2")
FIBER TYPE	NOVOMESH 950	SPACING	18" EACH WAY
DOSAGE (PER CUBIC YD OF CONCRETE)	10 lb	NOTE: TANK LID MAY USE FIBER REINFORCEMENT AS PER AT LEFT; HOWEVER, REBAR MUST ALSO BE USED PER ABOVE IN THE LID.	

REBAR SCHEDULE FOR 1000 S-T & 1000 P-T (3' OF SOIL COVER, WATER TABLE AT GROUND LEVEL & H520-44 TRAFFIC LOADS)			
SIDE WALLS		END WALLS	
HORIZ. DIAMETER	#4 (1/2")	HORIZ. DIAMETER	#4 (1/2")
HORIZ. SPACING	18"	HORIZ. SPACING	18"
VERT. DIAMETER	#4 (1/2")	VERT. DIAMETER	#4 (1/2")
VERT. SPACING	18"	VERT. SPACING	18"
FLOOR		LID (THICKNESS 7")	
DIAMETER	#4 (1/2")	DIAMETER	#5 (5/8")
SPACING	18" EACH WAY	SPACING	12" EACH WAY

NOTES:
1. ALL REINFORCEMENT IS TO BE ASTM A615 GRADE 60 WITH A MIN. OVERLAP OF 30 BAR DIAMETERS AT SPLICES (E.G., 15" FOR NO. 4 BAR)
2. UNLESS NOTED OTHERWISE, FLOOR AND WALL REBAR IS TO BE CENTERED IN THE CONCRETE MEMBER WITH THE HORIZONTAL REBAR TOWARDS THE OUTSIDE OF THE TANK.
3. FOLLOW ALL MANUFACTURER RECOMMENDATIONS FOR NOVOMESH 950 FIBER.



TANK SIDE WALL DETAIL



TANK END WALL DETAIL

PAGE: 5

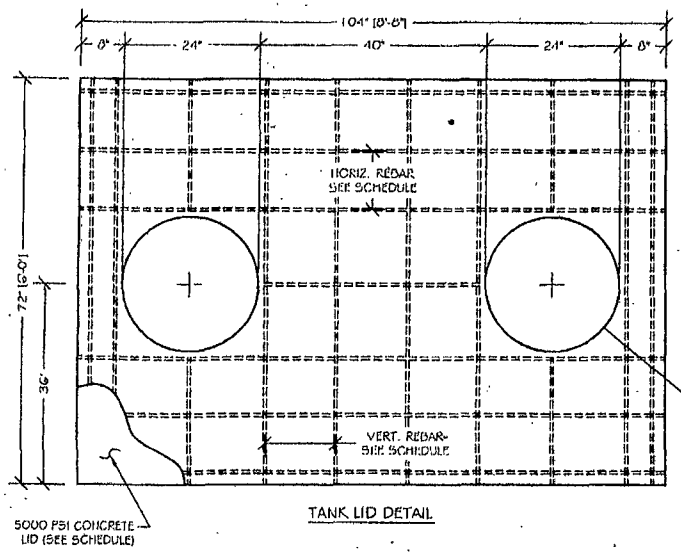
TANK CONSTRUCTION & RE-BAR SCHEDULE FOR MODELS
1000 S, 1000 S-T, 1000 P & 1000 P-T
HAGERMAN PRE-CAST

DATE: 2/2/12
DRAFTED BY: D.R.N.
SCALE: NTS

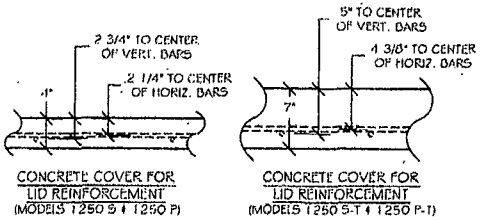
EK ENGINEERING INC.
P.O. BOX 3097 BATTLE GROUND, WA 98604
PHONE: (360) 687-7668 FAX: (360) 687-7669

Page 2 of 8

H ERMAN PRE-CAST
Bob & Anita Hagerman
PO Box 2842
Poulsbo WA 98370
360-598-6121 / 360-509-3702

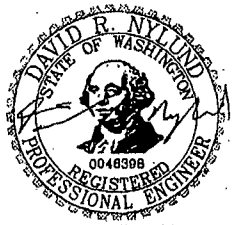


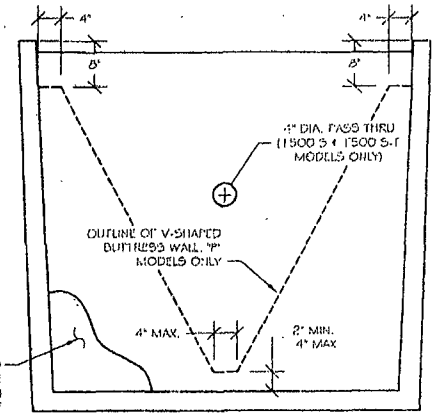
TANK LID DETAIL



CONCRETE COVER FOR LID REINFORCEMENT (MODELS 1250 S & 1250 P)

CONCRETE COVER FOR LID REINFORCEMENT (MODELS 1250 S-T & 1250 P-T)



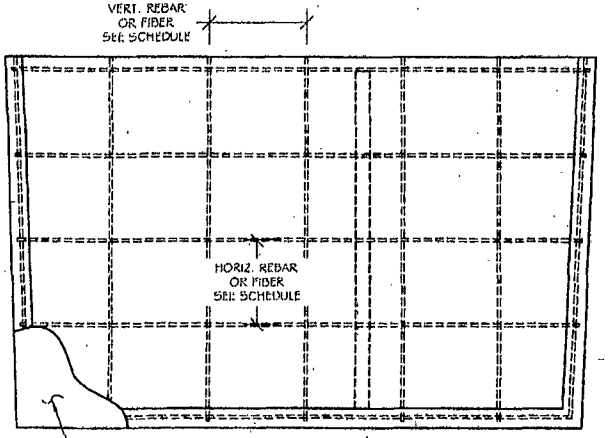


INTERCOMPARTMENTAL WALL DETAIL
(BUTTRESS WALL FOR 1500 P & 1500 P-T MODELS)

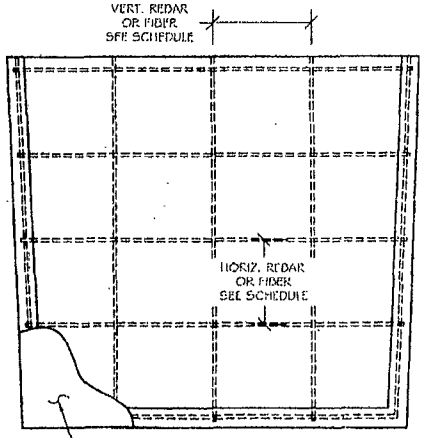
REINFORCEMENT SCHEDULE FOR 1250 S & 1250 P (3' OF SOIL COVER, WATER TABLE AT TOP OF TANK, 2,500 lb WHEEL LOAD)			
SIDE WALLS, END WALLS, INTERCOMPARTMENTAL WALLS, AND FLOOR		LID (THICKNESS 4")	
		DIAMETER	#4 (1/2")
FIBER TYPE	NOVOMESH 950	SPACING *	18" EACH WAY
DOSAGE (PER CUBIC YD. OF CONCRETE)	10 lb	NOTE: TANK LID MAY USE FIBER REINFORCEMENT AS LISTED AT LEFT; HOWEVER, REBAR MUST ALSO BE USED PER ABOVE IN THE LID.	

REBAR SCHEDULE FOR 1250 S-T & 1250 P-T (3' OF SOIL COVER, WATER TABLE AT GROUND LEVEL & HS20-44 TRAFFIC LOADS)			
SIDE WALLS		END WALLS	
HORIZ. DIAMETER	#4 (1/2")	HORIZ. DIAMETER	#4 (1/2")
HORIZ. SPACING	17"	HORIZ. SPACING	18"
VERT. DIAMETER	#4 (1/2")	VERT. DIAMETER	#4 (1/2")
VERT. SPACING	18"	VERT. SPACING	18"
FLOOR		LID (THICKNESS 7")	
DIAMETER	#4 (1/2")	DIAMETER	#5 (5/8")
SPACING	18" EACH WAY	SPACING	12" EACH WAY

NOTES:
1. ALL REINFORCEMENT IS TO BE ASTM A615 GRADE 60 WITH A MIN. OVERLAP OF 30 BAR DIAMETERS AT SPLICES (E.G., 15" FOR NO. 4 BARS).
2. UNLESS NOTED OTHERWISE, FLOOR AND WALL REBAR IS TO BE CENTERED IN THE CONCRETE MEMBER WITH THE HORIZONTAL REBAR TOWARDS THE OUTSIDE OF THE TANK.



TANK SIDE WALL DETAIL



TANK END WALL DETAIL

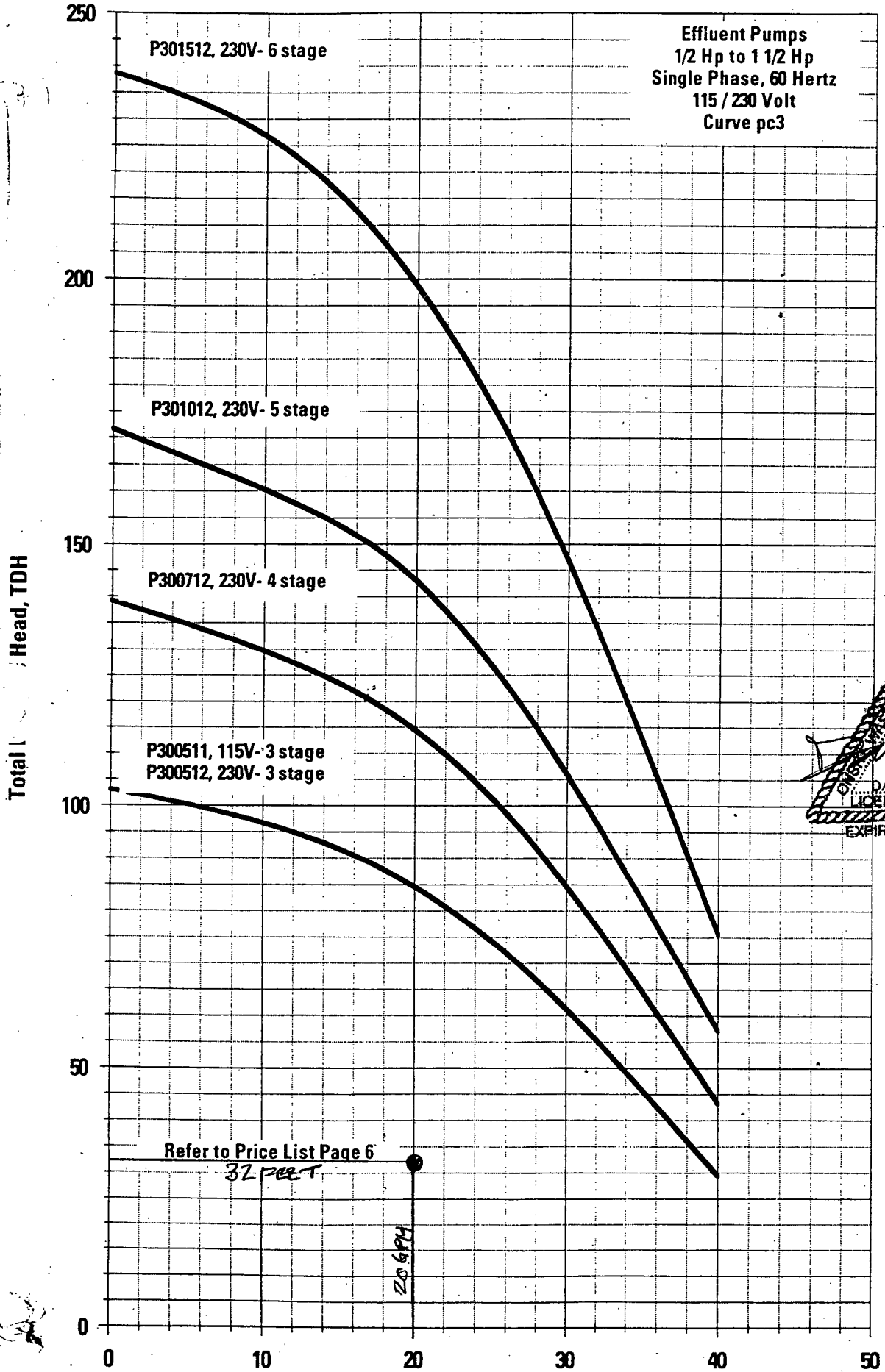
PAGE: 5

TANK CONSTRUCTION & RE-BAR SCHEDULE FOR MODELS
1250 S, 1250 S-T,
1250 P & 1250 P-T
HAGERMAN PRE-CAST

DATE: 6/23/11
DRAFTED BY: D.R.N.
SCALE: NTS

EK ENGINEERING INC.
P.O. BOX 3097 BATTLE GROUND, WA 98604
PHONE: (360) 687-7668 FAX: (360) 687-7669

Rated 3 of 8



Orenco Systems®
Incorporated

814 AIRWAY AVENUE

SUTHERLIN, OREGON

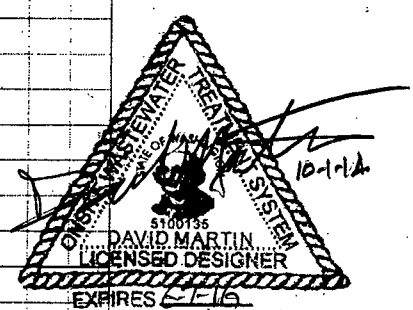
97479-9012

TELEPHONE:

(541) 459-4449

FACSIMILE:

(541) 459-2884



10-1-14

ETD-PU-PC-3
Rev. 12 9/97

Bremerton-Kitsap County Health District

Pressure Distribution On-Site Sewage Disposal System Work Sheet

Name of Applicant DUPRESNE Site Address or Location 11131 ROLLING BAY LANE - BAINBRIDGE ISLAND
Tax Assessors # 4156-001-004-0909 Date 9-30-2014
Designer's Name DAVID MARTIN Company Name MBH2O Phone # 360 930 3505

Property Information:

Number of Bedrooms 2
Maximum Daily Flow ..... 480 GPD
Total Absorption Area Required (Sq.Ft.)..... 480 SF

Trench Information: BISF

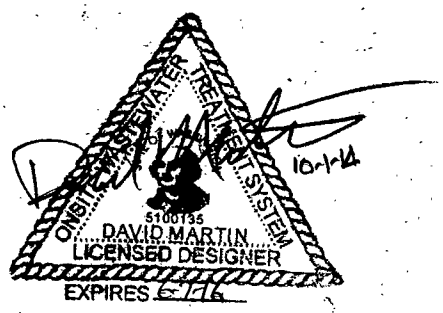
Trench Width ..... 10FT.
Trench Depth ..... 8FT.
Total Lineal Trench Length ..... 48FT.
Total Washed Drain Rock Under Lateral ..... 6IN.
Gravelless Chamber Drainfield (Yes- No)..... NO - POTENTIAL CONSIDERATION

Manifold, Lateral & Transport Information:

Total Lateral Pipe Length 184LF
Lateral Spacing ..... 24IN CENTERS
Transport Line Length ..... ±31LF
Transport Line Pipe (Schedule) 40
Manifold & Laterals Pipe (Schedule) 40
Lateral Line Diameter 2IN
Manifold Line Diameter 1IN
Transport Line Diameter 1IN

Pump Information:

Residual Lateral Head (Squirt Height) ..... 5'
Pump Size 20GPM 32PSI



Dosing Information:

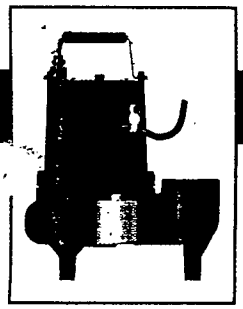
Orifice Spacing (inches)..... 48IN.
Total Orifices 46
Orifice Diameter ..... 1/8IN.
Total Dose Volume 20
Number of Doses Per Day 12
Size of Pump Tank (gallons) 1250

Control Panel Information :

PROGRAMMABLE TIMER REQUIRED

Other Information :

Plug
6 of 8



SKV50 SUBMERSIBLE SEWAGE EJECTOR PUMP

The Hydromatic SKV50 submersible pump is specifically designed to meet the demands of residential wastewater and sewage applications, and the quality standards of the professional plumber. The 2 inch NPT discharge pump (3 inch discharge optional) is available with a powerful 1/2 HP motor, in both automatic and manual configurations. The SKV50 can handle capacities up to 180 gallons per minute and heads to 20 feet.

The SKV50 features are impressive. The heavy-duty cast iron and engineered thermoplastic construction provides durability in rugged applications. The pump's high-capacity, recessed, vortex impeller is threaded to a stainless steel shaft providing long life in demanding applications. The recessed impeller is capable of passing 2 inch spherical solids and lint.

The SKV50 is offered in two automatic configurations. The SKV50AD uses a diaphragm piggyback switch to operate the pump automatically. The SKV50AW uses the wide angle mechanical float switch shown below.

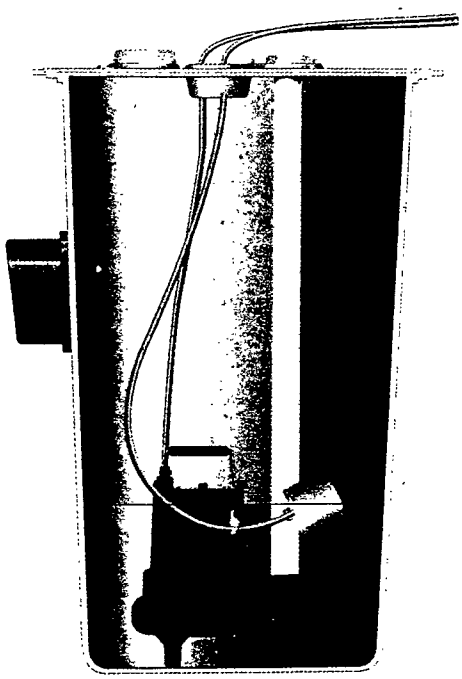
The SKV50's oil-filled motor provides superior cooling and lubrication, allowing the motor to run cool, quiet and trouble free for years. This oil-filled design also provides permanent lubrication of the shaft bearings, minimizing maintenance and extending the life of the pump. In addition, to protect against overheating and costly repairs, the motor windings contain an automatic reset thermal overload.

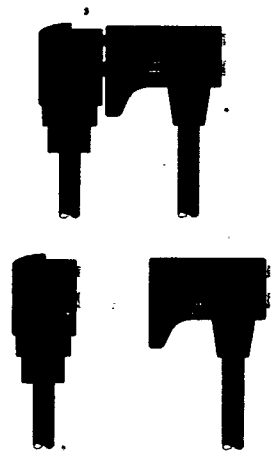
SKV50 TYPICAL INSTALLATION

Wide-angle Mechanical Piggyback Float Switch

The SKV50 uses a tilt-sensitive mechanical wide-angle float switch, which is hermetically sealed inside a non-corrosive polypropylene float. As liquid level rises, float changes angle until the switch makes circuit. The switch is not sensitive to rotation (no "up" side), which simplifies installation. Its reliability is proven to 500,000 cycles.

The wide-angle system operates between 115 and 125 degrees. Drawdown range is 6 to 14 inches, variable with length tethered to pump (5 inch maximum); and can be adjusted in seconds. Pumping differentials up to 35 inches can be obtained when the float is tethered to the discharge pipe. For maximum pump life, adjust tether length for deepest drawdown.





Piggyback Switch Plug

The Hydromatic wide-angle float switch features a unique piggyback plug arrangement. The pump power cord plugs into the back of the switch plug to provide automatic operation. To operate the pump manually, simply plug the pump power cord directly into the electrical outlet, bypassing the switch plug. The piggyback plug provides ease of service and allows the pump to be cycled manually on a periodic basis to ensure proper operation.

KITSAP COUNTY BOARD OF HEALTH ORDINANCE 2008-01

ONSITE SEWAGE SYSTEM AND GENERAL SEWAGE SANITATION REGULATIONS

Policy #24: Media Standards for All Media Based Treatment Components

Effective Date: April 1, 2011

Purpose: The purpose of this policy is to establish specific media standards for the sand media used in all media based treatment components.

Media Standards

1. Sand Media Specification

1.1. Non-Proprietary Treatment Components:

The filter media must meet items the Course Sand Media Specifications of a, b, and c, below:

a. Particle size distribution

Sieve	Particle Size	Percent Passing
3/8 in	9.50 mm	100
No. 4	4.75 mm	95 to 100
No. 8	2.36 mm	80 to 100
No. 16	1.18 mm	45 to 85
No. 30	0.6 mm	15 to 60
No. 50	0.3 mm	3 to 15
No. 100	0.15 mm	0 to 4

b. Effective Particle Size (D10) > 0.3 mm.

c. Uniformity Coefficient (D60/ D10) < 4.0

1.2. Proprietary Treatment Components:

The filter media must meet the minimum requirements of the manufacturer.

APPENDIX "A" TABLE 1

OPERATION AND MAINTENANCE INSPECTION SCHEDULE REQUIREMENTS

(ADDITIONAL OR LESS MAINTENANCE AND SAMPLING MAY BE REQUIRED BY THE HEALTH OFFICER.)

REQUIREMENTS FOR "AREAS OF SPECIAL CONCERN" (Type A)

MAINTENANCE	SAND FILTER	MOUNDS	PRESSURE	GRAVELLESS	AEROBIC*	GLENDON	T.S. 1	T.S. 2	T.S. 1 & 2 WITH DISINFECTION.
SEPTIC TANK (Including effluent screens) Yearly inspections.	X	X	X	X	X Including trash traps.	X	Pump if needed.	X	
DOSING TANK Yearly inspections.	X	X	X			X	Pump if needed.		
PUMP-Switches, floats, alarm system: Inspect and test yearly.	X	X	X			X	At 6 weeks and every 6 months.		
PUMP & PUMP SCREEN Yearly inspections and cleaning.	X	X	X		X	X			
SEEPS AROUND FILL Yearly inspections and cleaning.		X				X			
VISUAL INSPECTION INTO PORTS Yearly.	X	X		X		X			
YEARLY SQUIRT TEST Check head pressure.	X	X	X						
SAMPLE COLLECTION BOD, TSS, F. COLIFORM Yearly starting at 3-6 months.	X				X	X	X	X	At 6 weeks, fecal c. or free chlorine residual every 3 mo.

REQUIREMENTS FOR ALTERNATIVE SYSTEMS (Type B)

MAINTENANCE	SAND FILTER	MOUNDS	PRESSURE	GRAVELLESS	AEROBIC*	GLENDON	T.S. 2 WITH DISINFECTION: ADDITIONAL SAMPLING REQUIRED.
SEPTIC TANK (Including effluent screens) Yearly inspections.	X	X	X	X	X Including trash traps.	X	
DOSING TANK Yearly inspections.	X	X	X			X	
PUMP-Switches, floats, alarm system: Inspect and test yearly.	X	X	X			X	
PUMP & PUMP SCREEN Yearly inspections and cleaning.	X	X	X		X	X	
SEEPS AROUND FILL Yearly inspections.		X				X	
VISUAL INSPECTION INTO PORTS Yearly.	X	X		Yearly. Reduced DF is 2X/ yr. for the 1st 4 yrs. then 1X/ yr.		X	
YEARLY SQUIRT TEST Check head pressure.	X	X	X				
SAMPLE COLLECTION BOD, TSS, FECAL C. 1st sample 3-6 mo after start up-2nd sample 1 yr after 1st sample.	X				X	X	At 6 weeks, fecal c. or free chlorine residual every 3 mo.

*Use this chart for aerobic and other proprietary devices unless otherwise specified by the manufacturer.

GeoEngineers

8410 154th Avenue NE, Redmond, WA 98052. Telephone: (425) 861-6000, Fax: (425) 861-6050 www.geoengineers.com

MEMORANDUM

TO: Paul Faget, Swenson Say Faget; Rob Smallwood, Smallwood Design and Construction; Mike Fleck; Dick Puariea, McDowell NW Piling; Mike Davis, Davis Construction

FROM: Jack Tuttle Bcc Barbara Lr. Jack Tuttle

DATE: June 25, 2004

FILE: 6647-001-03

SUBJECT: Results of Boring Drilled on June 23, 2004 for Confirming Retaining Wall Design and Construction Assumptions

The log of the boring drilled on June 23, 2004 along the planned retaining wall alignment on the Fleck Rolling Bay Walk property and a site plan are attached. The boring was drilled to a depth of 29 feet using hand portable equipment set in a relatively level area on the slope above the residence. The boring location is about 15 feet south of the southwest corner of the residence, and about 12 vertical feet above the level of the boardwalk. A piezometer was installed in the boring to allow subsequent groundwater level measurements.

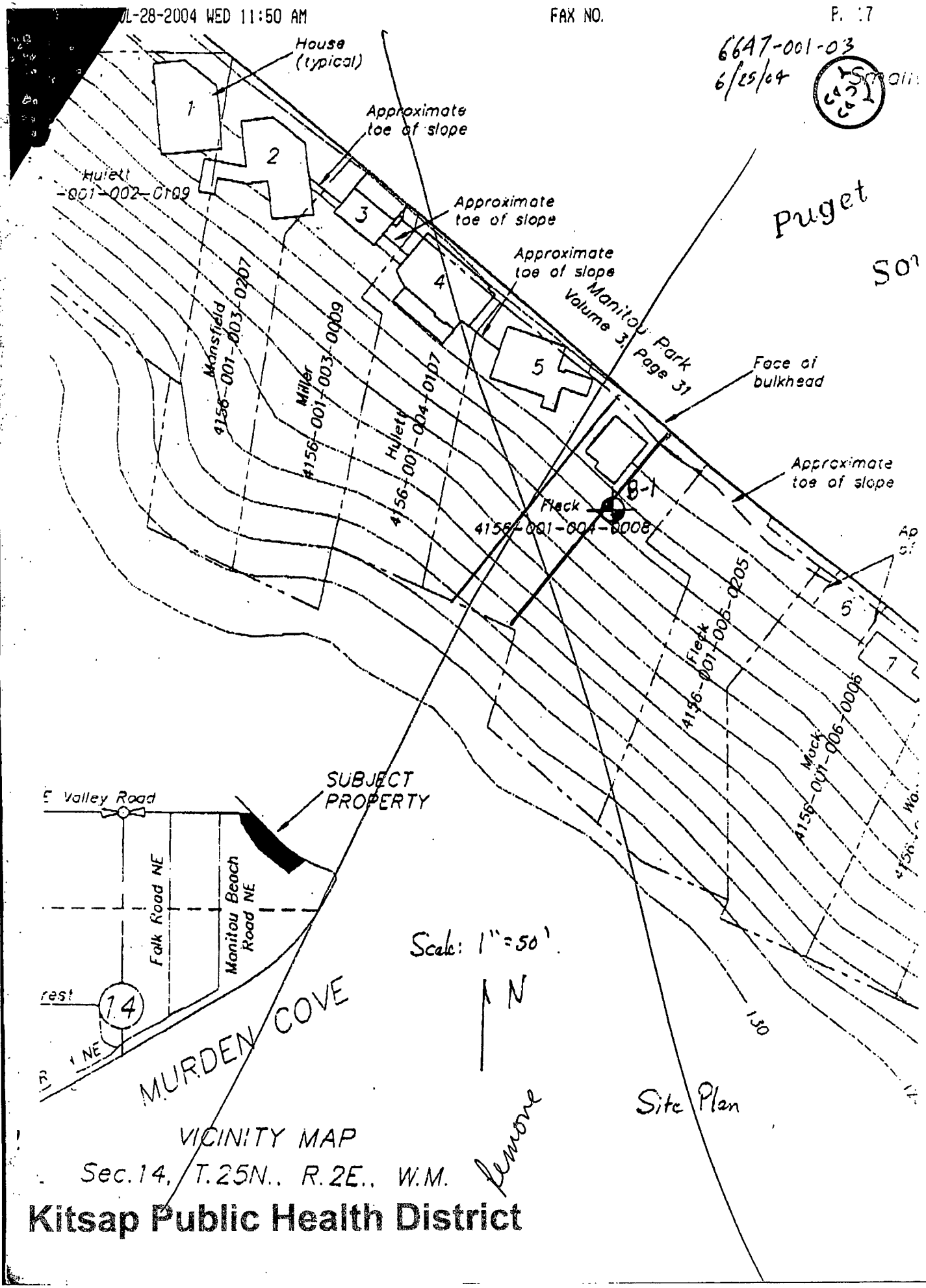
Our boring encountered 10 feet of medium dense sand with silt and gravel over dense to very dense sand and silty sand. We interpret the upper material to be either colluvium or debris from past slides. Prior to sampling at a depth of 17.5 feet, we noted approximately one foot of heave in the borehole. The depth at which we observed groundwater during drilling was also 17.5 feet. The driller added bentonite mud to control heave for the remainder of the boring. We measured the groundwater level at 17.7 feet in the piezometer on June 24, the day after the boring was completed. We expect that the groundwater level will fluctuate in response to tidal fluctuations and seasonal variations in precipitation.

The soil and groundwater conditions encountered in our recent boring are similar to those encountered in the boring that Myers Biodynamics Inc. drilled in 2002 behind the Benjamin Wood house (now owned by the Flecks). The Wood boring encountered 5 feet of loose sand and gravel fill and colluvium soils. From 5 to 20 feet, this boring encountered loose to medium dense fine sand with varying amounts of silt. Below 20 feet, the boring encountered dense sand with varying amounts of silt and gravel. Groundwater was encountered during drilling at a depth of 13.5 feet. The driller who drilled the Wood boring is the same one who drilled our recent boring above the Fleck residence; he advised us that he had also encountered heave while drilling the Wood boring.

We provided you with design parameters for the retaining walls in a letter dated September 24, 2003 and in a memorandum transmitted on July 24, 2003 (which is inadvertently dated March 24, 2003). Based on comparison of the boring logs, we conclude that the design parameters we provided previously remain applicable.

We trust that this information meets your present needs. If you have any questions concerning the information presented herein, please contact us.

6647-001-03
6/25/04



VICINITY MAP
Sec. 14, T. 25N., R. 2E., W.M.

Site Plan

remove

Kitsap Public Health District

Generalized Subsurface Conditions

Site subsurface conditions were explored by performing one subsurface boring exploration (B-1) on July 8, 2002. The general location of the boring exploration is shown on Figure 1. Drilling was conducted using a portable "Acker" drill rig due to limited site accessibility. The boring was positioned to sample soil at the location of the proposed retaining wall. Soil samples were obtained at 2-1/2 to 5 foot sample intervals using Standard Penetration Test (SPT) methods. A log of the boring is presented in Appendix A attached to this report along with a discussion of site exploration activities. Interpretations of subsurface soil and groundwater conditions are presented on the Generalized Subsurface Cross Section A-A' shown on Figure 2. Descriptions of soil and groundwater conditions observed in the explorations are presented below.

Soil

Subsurface soil conditions encountered in boring B-1 generally coincide with soil exposures observed in localized areas on the steep coastal slope. Site observations confirm coastal zone and geologic reference mapping interpretations that indicate the steep coastal slope is underlain by stratified sand and gravel deposits.

Soils observed in the boring exploration were generally composed of stratified intervals of sand with variable gravel and silt constituents, sandy gravel, silty sand, sandy silt, and an occasional hard clay layer as shown on Figure 2 and in the boring log presented on Figure A-1. The upper approximately 5 feet of soil was loose and moist, with trace organics and shell fragments interpreted to be a combination of backfill material used for construction of the previous wall that was likely obtained from the beach and reworked soil from past slope erosion/landslide activity. From approximately 5 to 20 foot depth, loose to medium dense, clean to silty fine sand was encountered. Below 20-foot depth, dense sand encountered with varying levels of silt and gravel.

Groundwater

Groundwater was observed at a depth of 13-1/2 feet below the existing site grade at the time of drilling. Soil above 13-1/2 feet was generally in a very moist condition. Below 13-1/2 feet, soil samples were generally wet.

Geotechnical Engineering Recommendations

Geotechnical engineering recommendations presented in the following sections are based on the results of the boring exploration and our understanding of the proposed retaining wall project at the time this report was prepared. The following sections discuss site preparation and excavation, drainage considerations, wall design alternatives, footing foundations, wall lateral earth pressures, and wall lateral resistance. As previously described, this report does not address the landslide risk

(42)

North wall above Fleck residence

When I arrived, a crew from McDowell NW Piling was installing the first soldier pile (pile NU-12) for the upper wall. The pile hole had just been drilled to a depth of approximately 32 feet using a large track-mounted Lodrill rig. The hole encountered 3 feet of gravel with sand, then fine to medium sand to the bottom. At our recommendation, the bottom pocket for the lower row of tiebacks was set about 2 feet vertically above the top of the lower wall. This allows more freeboard for catchment, and better access for tieback installation and testing.

During the day, McDowell also installed soldier piles NU-11 through NU-6 (see site plan excerpt). The holes stayed dry and open without casing. Little caving was observed. Soil conditions in these holes were similar to those of the first hole; only the depth to the top of the fine to medium sand varied from 3 to 17 feet.

VICINITY OF UPPER WALL NORTH OF SUBJECT LOT

0 - 32 Feet MEDIUM TO FINE SAND
[W/ 3' DISTURBED SOIL @ TOP]
NO WATER TO 32 FEET

Load testing was completed by 235P. All of the SL anchors were locked off at 25 percent of design load (11.25 kips).

We understand that McDowell will start soldier pile installation for the upper south wall later today or early tomorrow.

North walls above Fleck residence

McDowell completed installation of the upper wall soldier piles NU-1 through NU-5 today (see site plan excerpt). The holes were dry, and only minor caving of the sidewalls occurred. Soil conditions in these holes were similar to those encountered yesterday. The holes generally encountered 13 to 14 feet of gravel with sand and then fine to medium sand to the bottom of the holes.

Davis Construction plans to start installation of anchors for the upper wall later tomorrow or Monday September 27. They will prepare for this activity by reconfiguring the drill rig and cutting larger holes in the soldier piles for the anchors.

Other items

I discussed the status of the open cut between the two wall sites (Fleck and former Wood house) with Ivan of Northridge Construction. We recommend that the cut face be protected after wall construction is completed by a combination of a berm (quarry spalls or similar material) and plastic sheeting. Ivan indicated that he would also remove overhanging vegetation above the cut face and place ditches with check dams in the construction access road below to channel surface water. Ivan will discuss these features with Andrew Dee/Smallwood Design and Cosntruction.

GEOENGINEERS

FROM "REPAIR DESIGN" ON
ADJACENT PARCEL (EXISTING HOUSE)
TO NORTH
(1 OF 2 PAGES)

25 YEARS OF EXCELLENCE

May 16, 2005

Smallwood Design and Construction, Inc.
P.O. Box 11308
Bainbridge Island, WA 98110

Attention: Rob Smallwood

Subject: Geotechnical Consulting Services
Interaction of Septic Drain Field and Soldier Pile Walls
Fleck Property at 10994 Rolling Bay Walk
Bainbridge Island, Washington
File No. 6647-002-01

INTRODUCTION

This letter summarizes our evaluation of the potential impact of the operation of a septic drain field located on the bench between the two soldier pile walls on the property owned by Mr. and Mrs. Mike Fleck at 10994 Rolling Bay Walk in Bainbridge Island, Washington. You requested this letter in a telephone discussion with Bert Pschunder of GeoEngineers, Inc. on April 27, 2005.

We understand that the septic drain field on the property will occupy approximately 240 square feet on the level access bench which was created to provide access for construction of the upper slide debris catchment wall recently completed to protect the residence. The level bench is roughly 15 feet wide and confined on the lower side by a retaining wall approximately 5 feet high. This wall was designed to provide support for the construction equipment and to provide access for removal of slide debris as needed in the future. The lower wall consists of a cantilever soldier pile wall with tiebacks and pressure treated timber lagging. The upper slide catchment wall is intended to reduce the risk of property damage and personal injury for the residence, and acts independently of the lower wall.

We also understand that the drain field, which is being designed by others, will be installed sufficiently deep to expose the underlying dense sand formation. The system will be designed to have a dosage rate of 20 gallons every two hours, or about 240 gallons per day. The Kitsap County Health Department has expressed concern that seepage from the drain field will flow toward the timber lagging and possibly cause deterioration of same.

EVALUATION AND CONCLUSIONS

Based on our observations during construction of the upper and lower walls, the soils at or near the bench surface along the face of the upper wall consist of dense advance outwash sand. The surface of the natural soil slopes downward toward the shore line such that fill was required to form the bench to the width constructed. This fill consisted of generally loose sand and colluvial materials. The thickness of the fill increases from nearly zero at the upper wall to perhaps as much as 8 to 10 feet at the back side of the lower wall. If the native soils are exposed at the bottom of the prepared drain field or if clean granular fill is placed from the surface of the undisturbed soils to the base of the drain field, the seepage from the drain field is expected to infiltrate into the native sand deposit with little or no lateral movement, particularly considering the design dosage rate. It is possible that some flow could follow the sloping surface of the dense sand to and beneath the lower wall, and possibly be evidenced at the toe of the slope. However, we have noted no

Earth Sciences & Technology
8410 154th Avenue NE
Redmond, WA 98052
telephone 425.861.6000
facsimile 425.861.6050
website www.geoengineers.com

spring activity on or at the toe of this portion of the slope following heavy rains during our numerous visits to observe conditions along Rolling Bay Walk in the past eight years.

Seepage from the drain field will have no effect on the upper wall since the gradient of flow will be toward Puget Sound.

Pressure-treated timber lagging was installed for both the lower and upper walls. A minimum preservative retention rate of 0.4 pounds per cubic foot was also specified. Such treatment usually results in a life span of several decades for the timber. We do not expect the moisture conditions in the soils being retained by the lagging to be materially different from the influence of any seepage from the drain field than it will be for saturation of these soils by heavy rainfall.

In any event, if deterioration of the lagging in the lower wall should occur in the future it can be replaced without affecting the function of the upper wall. If any such deterioration does occur, it could be easily mitigated by replacing the lagging and installing a prefabricated drain to collect the seepage and route it down 2 feet below the base of the lagging.

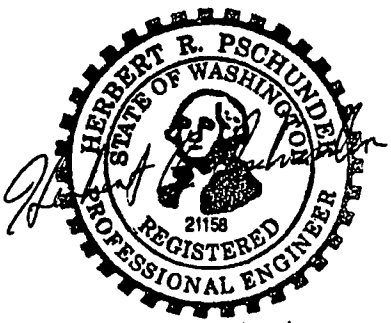
Installation of a 10-foot wide drain field will require excavation relatively close to the two retaining walls. We recommend that the excavation along the upper wall be done in sections such that the length of the full depth of excavation is not longer that the distance between soldier piles (8 feet). The drain pipe should be laid and backfill placed and compacted up to the original grade of the access road for a width of at least 5 feet as measured out from the face of the wall before extending the excavation further.

Excavation along the back of the lower wall can be done as close to the wall as practical without dislodging the lagging. The depth of the excavation must be controlled to avoid contact with the underlying tiebacks. The closer the excavation is to the back of the wall the higher the probability that lateral seepage from the drain field could daylight through or immediately below the wall lagging. To control this, we recommend that an impermeable membrane be placed on the face of the excavation (or on the back of the lagging if the timbers are exposed). The membrane should be extended down to the base of any impermeable soils placed for bedding around the drain pipe.

We trust that this information meets your present needs. If you have any questions concerning the information presented in this letter, please contact us.

Sincerely,

GeoEngineers, Inc.



5/16/05

Herbert R. Pschunder, P.E.
Senior Geotechnical Engineer

Jack K. Tuttle, P.E.
Senior Consultant

HRP:JKT:nh
Redm:\01\finals\664700201 L1.doc

KITSAP PUBLIC HEALTH DISTRICT

Environmental Health
345 6th Street, Suite 300
Bremerton, WA 98337
360-337-5235

Memo #: 136628
Date Applied: OCT 2 - 2014
Fee paid: N/C per SK
Clerks initials: BDM

DRINKING WATER / ONSITE SEWAGE WAIVER REQUEST FORM No. 1 of 2

Waiver Request From (Please check the following in regard to which Regulations are the subject of the waiver):

- ☒ **Local Septic Regulations (KCBOH Ordinance No. 2008A-1)**
- ☐ **Local Drinking Water Regulations (KCBOH Ordinance No. 1999-6)**

Section I. *(Completed by Applicant)*

(1) Name: MARGARET DUFRESNE

(2) Site Address: 11143 ROLLING BAY WALK N.E BAINBRIDGE ISLAND 98110

(3) Tax Parcel No.: 4156-001-004-0909

(4) Regulatory Requirement: 100 FOOT SETBACK FROM OSS DISPERSAL COMPONENT TO SURFACE WATERS

(5) Waiver Requested: REDUCE SETBACK TO 50 FEET

(6) Waiver Justification and Mitigation: 1- ONLY AVAILABLE AREA ON-SITE 2 - DISPERSAL EFFLUENT TO RECEIVE ADVANCED TREATMENT WITH AEROBIC TREATMENT FOLLOWED BY SAND FILTRATION WITH GREATER THAN 24" VERTICAL SEPARATION 3- OPERATION AND MAINTENANCE CONTRACT REQUIRED TO ASSURE LONG-TERM OPERATION 4- NO DRINKING WATER SOURCES WITHIN 100 FEET

Section II. *(Completed by Kitsap Public Health Officer)*

(7) Review Criteria: DOH CLASS A CRITERIA

(8) Mitigation Measures (in addition to those proposed in Section I.:

(9) Comments/Conditions of Approval: B+B TREATMENT, MTM REQUIRED 24"+ VERTICAL

(10) Type of Waiver: ☒ Class A ☐ Class B ☐ Class C ☐ Local

Section III. *(Completed by Kitsap Public Health Officer)*

This Waiver Request has been reviewed according to the applicable provisions of Chapter 246-272 WAC or KCBOH Ordinance No. 2008A-1 or 1999-6. The review criteria applied, and the mitigation measures proposed and/or required, have been evaluated for their ability to provide public health protection at least equal to that provided by the regulations.

This Waiver Request is: ☒ **Approved/Granted** (Subject to the above Conditions of Approval)

☐ **Denied**

☐ **Accepted for Non-Conforming Onsite Sewage System**

KPHD Health Office Signature: _____ Date: 10/9/14

KPHD Health Officer Name: JOHN KEISS

KITSAP PUBLIC HEALTH DISTRICT

Environmental Health
345 6th Street, Suite 300
Bremerton, WA 98337
360-337-5235

Memo #: 31b29
Date Applied: OCT 2 - 2014
Fee paid: $109
Clerks initials: BM

DRINKING WATER / ONSITE SEWAGE WAIVER REQUEST FORM No.2 of 2

Waiver Request From (Please check the following in regard to which Regulations are the subject of the waiver):

- ❑ **Local Septic Regulations (KCBOH Ordinance No. 2008A-1)**
- ❑ **Local Drinking Water Regulations (KCBOH Ordinance No. 1999-6)**

Section I. *(Completed by Applicant)*

(1) Name: MARGARET DUPRESNE

(2) Site Address: 11143 ROLLING BAY WALK NE · BAINBRIDGE ISLAND 98110

(3)Tax Parcel No.: 4156-001-004-0909

(4) Regulatory Requirement: 50 FOOT SETBACK FROM SEWAGE TANKS TO SURFACE WATERS

(5) Waiver Requested: REDUCE SETBACK REQUIREMENT TO 25 FEET

(6) Waiver Justification and Mitigation: 1-ALL TANKS WILL REQUIRE WATERPROOF SURFACE BARRIER 2-ALL TANK CONNECTIONS MUST HAVE FLEXIBLE COUPLINGS 3-TANKS MUST BE TESTED FOR WATERTIGHT CONSTRUCTION AFTER PLACEMENT 4-ALL TANKS TO HAVE WATERTIGHT RISERS TO FINISH GRADE WITH LOCKABLE LIDS

Section II. *(Completed by Kitsap Public Health Officer)*

(7) Review Criteria: DOH CLASS A CRITERIA

(8) Mitigation Measures (in addition to those proposed in Section I.:

(9) Comments/Conditions of Approval: M&M REQUIRED REGISTERED TANKS

(10) Type of Waiver: ☒ Class A ❑ Class B ❑ Class C ❑ Local

Section III. *(Completed by Kitsap Public Health Officer)*

This Waiver Request has been reviewed according to the applicable provisions of Chapter 246-272 WAC or KCBOH Ordinance No. 2008A-1 or 1999-6. The review criteria applied, and the mitigation measures proposed and/or required, have been evaluated for their ability to provide public health protection at least equal to that provided by the regulations.

This Waiver Request is: ☒ **Approved/Granted** (Subject to the above Conditions of Approval)
❑ **Denied**
❑ **Accepted for Non-Conforming Onsite Sewage System**

KPHD Health Office Signature: JOHN KEESS Date: 10/9/14

KPHD Health Officer Name: JOHN KEESS

11143 ROLLING BAY WALK NE, Bainbridge Island

CHRONOLOGICAL CONTROL SHEET

BSA- New

Applicant: **DUFRESNE MARGARET A**

TaxID: **41560010040909** **Lot: N/A** Memo: **318627** BP: DCD-LU:

Contractor: **MILLER BAY WATER COMPANY**

Flagged: **Other** **credit $109 to new BSA and/or associated waiver submittal, see JK** **10/1/2014**

RECEIVED ON	INITIALS	ACTION TAKEN / COMMENTS	ROUTE TO	DATE
10/2/2014	BSJ	Over the Counter; Expired BSAs in Laserfiche		10/02/2014
	JK	expiry KAD connectn - ok	MK	10/3
			BR	10/7

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RECEIVED ON	INITIALS	ACTION TAKEN / COMMENTS	ROUTE TO	DATE
10/2/2014	BSJ	Over the Counter; Expired BSAs in Laserfiche		10/02/2014
			SB	10/2
		Visited site in Sept. w/designer + JK for BSA Pre-App		
		No. soil logs dug		
10-21-14	SSB	Approved	BR	10/23/14
05/22/18	MC	Rcvd revision from Miller Bay OTC. Routed and put in daily work.		
5-24-18	SSB	Approved	BR	