Wintergreen Townhomes
Preliminary Plat
UTILITY REPORT

Bainbridge Island, Washington 98110

Prepared For

Central Highland Builders, Inc.
PO Box 2879
Poulsbo, WA 98370

Prepared by

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241 Ericksen Avenue
Bainbridge Island, WA 98110

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2/18/21

Project Description

The proposed project consists of constructing 74 townhouse units in 11 buildings on two parcels. Also included will be the parking, utility connections, and storm drainage mitigation to support the buildings. The two parcels are located on the west and east sides of Wintergreen Lane NE on Bainbridge Island.

STORMWATER DRAINAGE

Existing

The two subject properties were part of the Visconsi Master Plan. The Visconsi Master Plan consisted of five parcels totaling 8.3 acres. The proposed project is located on lots A and D. As part of the Visconsi Master Plan infrastructure was constructed to support development on each of the lots. This included water and sewer utility mains and services to each of the lots. In addition, a stormwater mitigation system was constructed on Lot D that consists of a stormwater treatment system of multiple Stormfilter treatment units and two sets of detention tanks. Also, a mitigation system was constructed on the west side of Lot B to mitigate for Wintergreen Lane and portions of Lots A and D. The proposed project is planning on utilizing the capacity of those systems to provide mitigation for the applicable areas.

Lot A is approximately 1.32 acres and Lot D is approximately 1.62 acres. Lot A slopes to the west reaching a low berm. Once at the top of the berm the site slopes to the west again. The grade steepens after leaving the property. When SR305 was constructed is was cut into this area. The slope is approximately 20 feet in height. At the bottom of the slope is the road side ditch along SR305.

Lot A is currently vegetated with pasture grass on the majority of the property. On the berm along the west side there are new plantings from the Visconsi Master Plan work. There are no significant trees on the parcel.

On Lot D the ground is very flat and slopes to the southeast. On the southeast portion of the property the ground slopes up approximately 3 feet and then is flat until reaching the property line. The northeast portion of the property there is a retaining wall that varies in height from 0 to 8 feet.

Lot D is currently vegetated with pasture grass on the majority of the property. Along the east edge of the property the vegetation consists of young plantings from the Visconsi Master Plan and established vegetation. A wetland is located in the northeast corner of the property. The existing drainage system on the site discharges through a dispersion into the 100 foot buffer of before reaching the wetland.

Included in the report is the drainage basins from the Visconsi Master Plan and the proposed basins from this development. These diagrams show how the proposed impervious areas will be divided into which mitigation system and that the mitigation systems were designed for larger impervious areas that is proposed with this development. Please note that Lot A did not have a drainage system constructed for development within its basin. For this reason a new mitigation is proposed.

As previously determined in the Visconsi Master Plan, soils on the site prevent the use of infiltration in accordance with the City's stormwater code.

Proposed

Lot A

The proposed project includes the construction of 7 buildings, walkways, an access lane, a parking area and landscape areas. The existing improvements on the property are located on the north, east and south perimeters. Along the north edge a new parking area will be constructed that ties into the Wintergreen Clinic parking area. Also, a garbage enclosure will be constructed. The existing improvements on the east side of the site will remain unchanged. A sidewalk will be added to the east frontage. Along the south side of the site the existing parking in the area will be reconfigured to allow for a new access way to be constructed.

On the east side of the property Wintergreen Lane runs south to north. The grade of Wintergreen Lane is to the north. Water from this area drains into a Stormfilter treatment system first and then into a underground detention system. The existing system was designed to accommodate 46,296 square feet of new impervious area and 3,647 square feet of replaced impervious surface. The proposed basin includes 43,394 square feet of new impervious area and 3,647 square feet of replaced impervious area. Based on this, the existing system has more capacity than is required by the proposed development.

The remainder of the site doesn't have an existing drainage system. We are proposing to construct 7,428 square feet of hardscape and 18,726 square feet of roof, and 12,027 square feet of landscaping. Of that hardscape 4,647 square feet will be for the proposed access way.

Threshold Analysis
The project will meet Minimum Requirements 1-9 as described by the City's stormwater code. See Table 1 for a description of how each minimum requirement is addressed.

Table 1
Summary of Stormwater Minimum Requirements

Minimum Requirement	Comment
1. Stormwater site plan	A stormwater site plan is presented.
2. Construction stormwater pollution prevention	A TESC plan will be developed as part of the development of the construction documents.
3. Source control of pollutants	All applicable source control best management practices will be implemented on site in accordance with City requirements.
4. Preservation of natural drainage systems and outfalls	Runoff will continue to discharge to the west and into the SR305 road side ditch.
5. On-site stormwater management	Disturbed soil with in landscape areas will be amended to meet the requirements of BMPT5.13. Soils will be amended individually as part of the construction of the buildings. Perforated stubout

	connections will be included in the building roof drain system. The runoff from the access way will be routed to an under drained rain garden.
6. Runoff treatment	The runoff from the access way will be routed to an under drained rain garden that will treat over 91% of the runoff from this area. Because the soils have been heavily modified with the development of the Visconsi Master Plan, they can not be relied upon to reliably infiltrate stormwater. Because of this, perforated under drains will be installed in the rain garden to collect the treated water. The under drain will discharge to a detention vault.
7. Flow control	A new detention system, consisting of two concrete vaults, will be constructed in the landscape space to the west of the east units. This system will reduce the peak 100 year flow from the project from 0.2717 cubic feet per second to 0.1138 cubic feet per second. In addition, the system was designed to meet the Flow Control Duration criteria as required by code.
8. Wetlands protection	Not applicable.
9. Operation and maintenance	An operation and maintenance manual and maintenance covenant will be provided.

Stormwater Mitigation

Lot B

The proposed project includes the construction of 4 buildings, walkways and landscape areas. The existing parking area will be reduced to accommodate this construction. The existing drainage system on the site was designed to accommodate 33,722 square feet of impervious area. The proposed project will construct a total of 31,968 square feet of impervious area. The flow control drainage system on Lot D was divided into two basins, south and north. The drainage system on the south basin was constructed to mitigate 19,876 square feet of impervious area. We are proposing to construct 18,686 square feet of impervious area that drains to this system. The drainage system on the north basin was constructed to mitigate 13,846 square feet of impervious area and we are proposing to construct 13,282 square feet. Based on this, the existing stormwater flow control systems have more capacity than needed.

The existing parking lot in the north basin will need to be graded to drain to 2 new stormfilter units to provide water quality treatment. The parking area in the south basin will continue to drain to the existing system without modifications.

Threshold Analysis
The project will meet Minimum Requirements 1-9 as described by the City's stormwater code. See Table 1 for a description of how each minimum requirement is addressed.

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1. Stormwater site plan	A stormwater site plan is presented.
2. Construction stormwater pollution prevention	A TESC plan will be developed as part of the development of the construction documents.
3. Source control of pollutants	All applicable source control best management practices will be implemented on site in accordance with City requirements.
4. Preservation of natural drainage systems and outfalls	Runoff will continue to discharge to the existing outfall that drains east.
5. On-site stormwater management	Disturbed soil with in landscape areas will be amended to meet the requirements of BMPT5.13. Soils will be amended individually as part of the construction of the buildings. Perforated stubout connections will be included in the building roof drain system.
6. Runoff treatment	The south existing stormfilter will remain. The north Stormfilter will be removed and replaced with two smaller Stormfilter systems to treat water from the pollution generating surfaces.
7. Flow control	Two detention systems exist on the property. The proposed impervious surface to these systems is less than what they were designed for. Based on this, the systems have adequate capacity to mitigate the runoff from the site.
8. Wetlands protection	The existing system will reduce the peak flows from the site. In addition, the outlet from the system will have to travel over 100 feet before it reaches the wetland. Based on this the project will not directly discharge to the wetland offsite.
9. Operation and maintenance	An updated operation and maintenance manual and maintenance covenant will be provided.

Stormwater Mitigation
The mitigation system for the project is divided into three systems. One system for the north portion of the lot, a system for the south portion of the lot and a system for the west portion of the lot. The north system will collect runoff from the roofs and landscaping and route it to the existing detention system located in the northeast corner of Lot D. The parking area in this portion of the property will be regraded to route pavement runoff to two small Stormfilter units.

The south system will also collect runoff from the roofs and landscaping to route it to the existing detention system located in the southeast corner of the property. The runoff from the existing parking and accessway will continue to drain to the existing drainage system. The outlet of the tank will be rerouted to avoid the proposed residences to the north and the second existing system.

It should be noted that the existing west parking area drains to either the water quality and quality mitigation systems on the Wintergreen Clinic site or the Walgreens site. These areas were addressed in the design of those systems.

ONSITE UTILITIES

Water
Existing
A 12-inch water line runs along the length of Wintergreen Lane and an 8-inch water line that runs from Wintergreen Lane to Polly's Lane. Domestic and fire services lines have been stubbed out to both lots in multiple locations. There is an existing fire hydrant south of Lot D and another near the entrance to the Franciscan Urgent Care.

Proposed
The project will extend the existing service lines to each of the buildings.

Sanitary Sewer
Existing
An existing 8-inch sanitary sewer main is located in Wintergreen Lane. 6 inch side sewers have been provided to the building sites.

Proposed
The 8-inch sanitary main will be extended to provide better access to the proposed units. These extensions will end in a manhole. From these points 6-inch diameter side sewers will extend to the individual units. Each side sewer will serve a maximum of 2 units.

Offsite Sanitary Sewer System
City Staff suspect that there may be a capacity issue in a portion of their sewer system downstream of our site. They have requested that we evaluate this issue taking into consideration the load from the proposed project. We are currently working with the City to obtain information on the existing system to perform that evaluation.

Other Utilities
Power, telephone and cable are located in Wintergreen Lane. Power, telephone, and cable service will be designed by others.

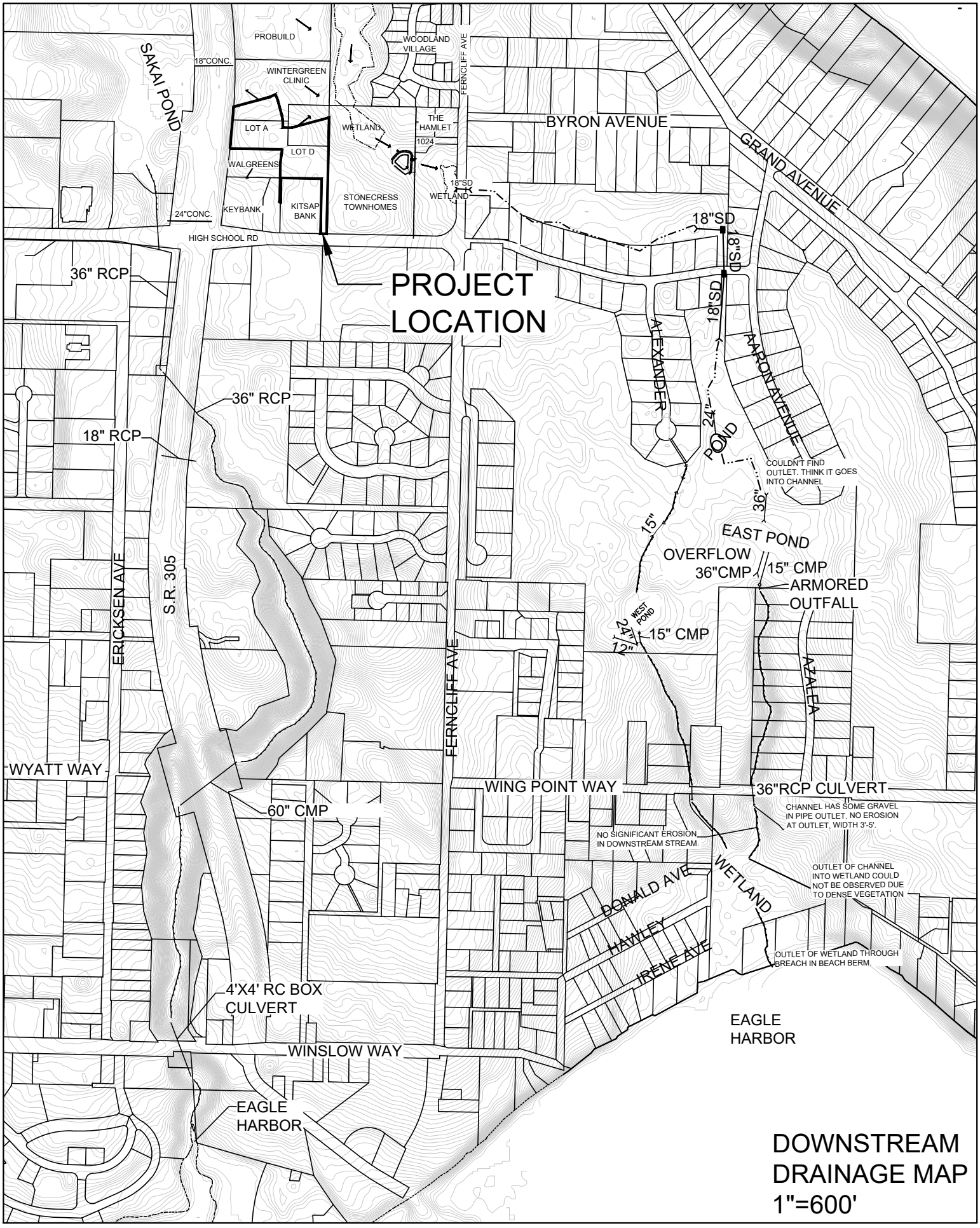
EROSION CONTROL

The Contractor will be responsible for maintaining erosion control facilities on the site during construction and for ensuring that sediment does not leave the site. The general principles of construction pollution prevention are:

- Retain native vegetation
- Prevent erosion rather than treat sediment laden water.
- Employ site specific best management practices (BMPs)
- Divert upslope runoff around disturbed area
- Phase construction operations to reduce total amount of disturbance at one time
- Amend soils before seeding
- Minimize the slope length and steepness of disturbed areas
- Reduce runoff velocities
- Prevent the tracking of sediment off site
- Employ BMPs that address not only erosion but also other potential pollutants.

A detailed erosion control plan will be submitted with the construction permit for review.

MAPS


PROBUILD
WOODLAND VILLAGE
FERNCLIFF AVE
SAKAI POND
18"CONC.
WINTERGREEN CLINIC
LOT A
WETLAND
THE HAMLET 1024
BYRON AVENUE
GRAND AVENUE
LOT D
WALGREENS
18"SD WETLAND
KEYBANK
KITSAP BANK
STONECRESS TOWNHOMES
24"CONC.
HIGH SCHOOL RD
18"SD
36" RCP
PROJECT LOCATION
ALEXANDER
AARON AVENUE
36" RCP
18" RCP
24"
POND
COULDN'T FIND OUTLET. THINK IT GOES INTO CHANNEL
36"
15"
EAST POND
OVERFLOW
36"CMP
15" CMP
ARMORED
OUTFALL
ERICKSEN AVE
S.R. 305
WEST POND
24"
12"
15" CMP
AZALEA
FERNCLIFF AVE
WYATT WAY
WING POINT WAY
36"RCP CULVERT
60" CMP
CHANNEL HAS SOME GRAVEL IN PIPE OUTLET. NO EROSION AT OUTLET, WIDTH 3'-5'.
NO SIGNIFICANT EROSION IN DOWNSTREAM STREAM.
OUTLET OF CHANNEL INTO WETLAND COULD NOT BE OBSERVED DUE TO DENSE VEGETATION
WETLAND
DONALD AVE
HAWLEY
IRENE AVE
OUTLET OF WETLAND THROUGH BREACH IN BEACH BERM.
4'X4' RC BOX CULVERT
EAGLE HARBOR
WINSLOW WAY
EAGLE HARBOR
DOWNSTREAM DRAINAGE MAP
1"=600'

ENGINEERING CALCULATIONS

WEST LOT
VALUES DO NOT INCLUDE IMPERVIOUS AREA TO REMAIN

TOTAL		47908		1.100 AC
NEW IMPERVIOUS		30119	SF	0.691 AC
	BUILDING	18726	SF	
	ROAD	5606	SF	
	PATH	5787	SF	
REPLACED IMPERVIOUS		42	SF	0.001 AC
	BUILDING	0	SF	
	ROAD	42	SF	
	PATH	0	SF	
VEGETATIVE BUFFER		5386	SF	0.124 AC
LANDSCAPING		12361	SF	0.284 AC

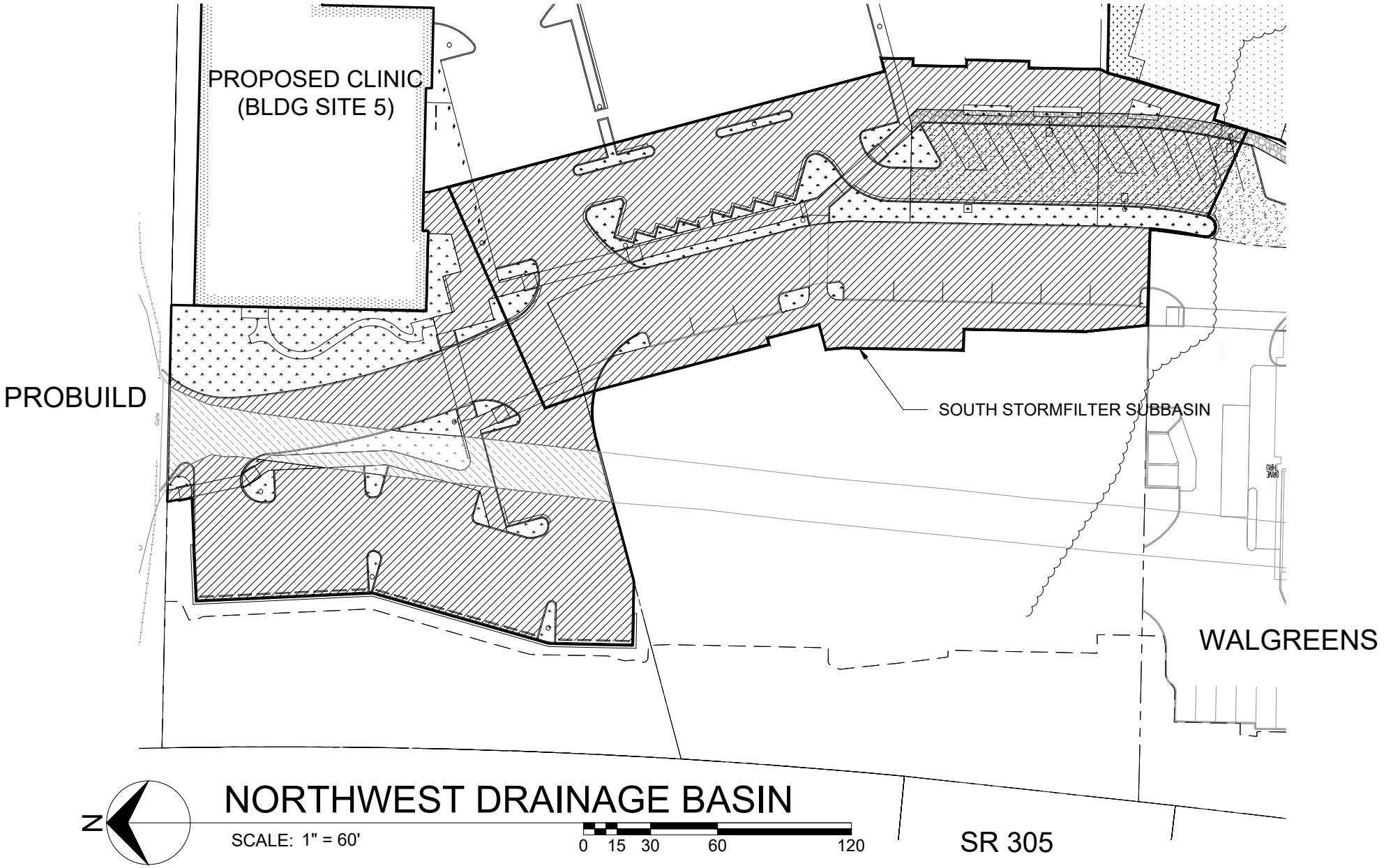
OFF PROPERTY			
NEW IMPERVIOUS	116	SF	0.003 AC
NEW PATH	38	SF	
NEW ROAD	78	SF	
REPLACED IMPERVIOUS			
ROAD	60	SF	0.001 AC
REMOVED IMPERVIOUS	60	SF	
LANDSCAPE	30	SF	0.001 AC

EAST LOT

VALUES DO NOT INCLUDE IMPERVIOUS AREA TO REMAIN

TOTAL	26348		0.605 AC
NEW IMPERVIOUS	13011	SF	0.299 AC
BUILDING	10219	SF	
ROAD	131	SF	
PATH	2661	SF	
REPLACED IMPERVIOUS	4456	SF	0.102 AC
BUILDING	3141	SF	
ROAD	79	SF	
PATH	1236	SF	
REMOVED IMPERVIOUS	1260	SF	0.029 AC
LANDSCAPING	7621	SF	0.175 AC

OFF PROPERTY			
NEW IMPERVIOUS	116	SF	0.003 AC
NEW PATH	38	SF	
NEW ROAD	78	SF	
REPLACED IMPERVIOUS			
ROAD	0	SF	0.000 AC
REMOVED IMPERVIOUS	60	SF	
LANDSCAPE	30	SF	0.001 AC



NORTHWEST DRAINAGE BASIN

N

SCALE: 1" = 60'

0 15 30 60 120

NEW IMPERVIOUS
AREA = 46296 SF

LANDSCAPING
AREA = 9365 SF

REPLACED IMPERVIOUS
AREA = 3647 SF

REMOVED IMPERVIOUS
AREA = 691 SF

NW DRAINAGE BASIN

WINTERGREEN MEDICAL BUILDING

PLAT UTILITY PERMIT SUBMITTAL

BROWNE • WHEELER
ENGINEERS, INC
241 ERICKSEN AVENUE NE
BAINBRIDGE ISLAND, WA 98110
P 206.842.0605 INFO@BrowneWheeler.COM

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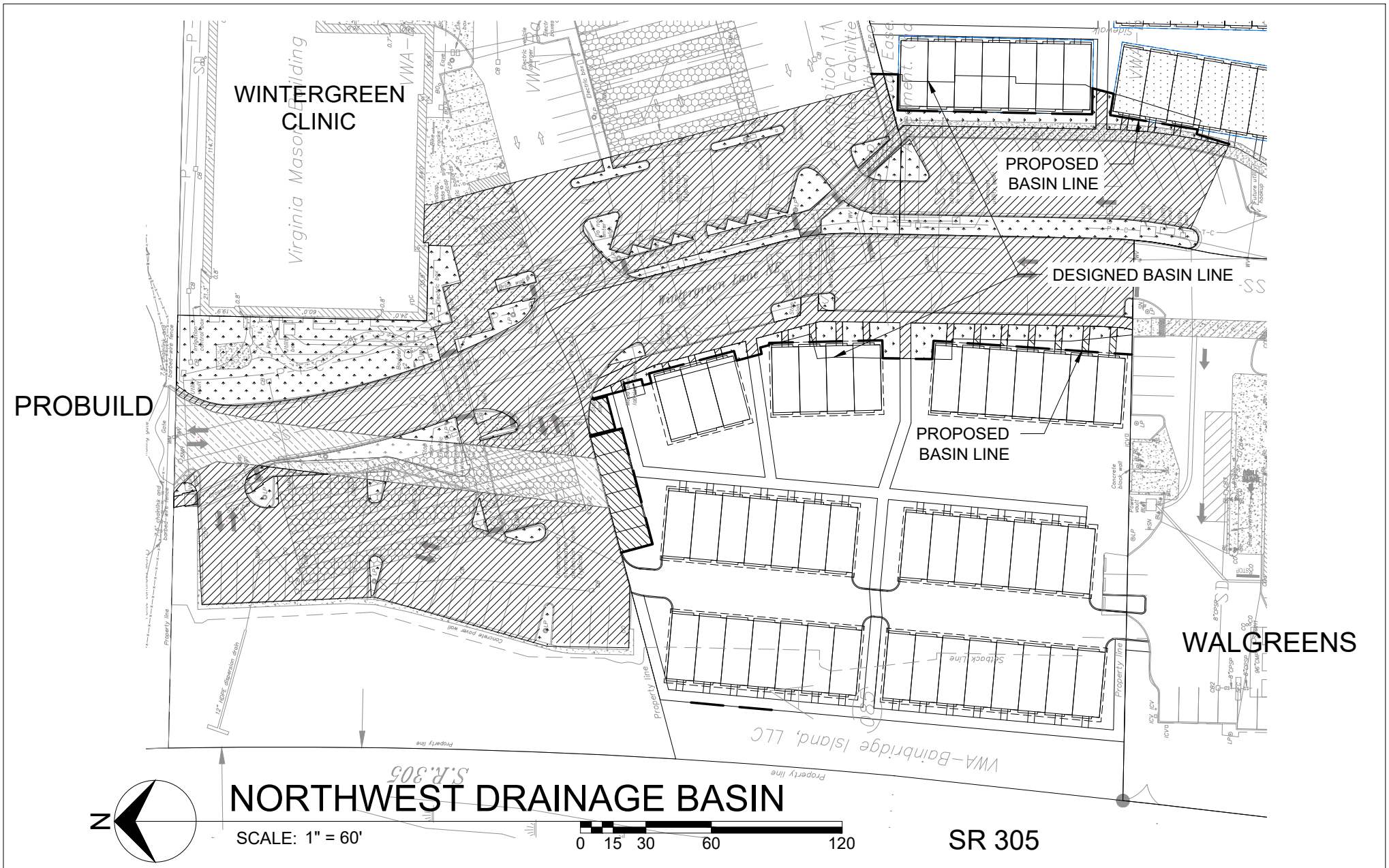

NORTHWEST DRAINAGE BASIN

NEW IMPERVIOUS
AREA = 43394 SF

LANDSCAPING
AREA = 11620 SF

REPLACED IMPERVIOUS
AREA = 3647 SF

REMOVED IMPERVIOUS
AREA = 691 SF

NW DRAINAGE BASIN

WINTERGREEN TOWNHOMES

PRELIMINARY PLAT SUBMITTAL

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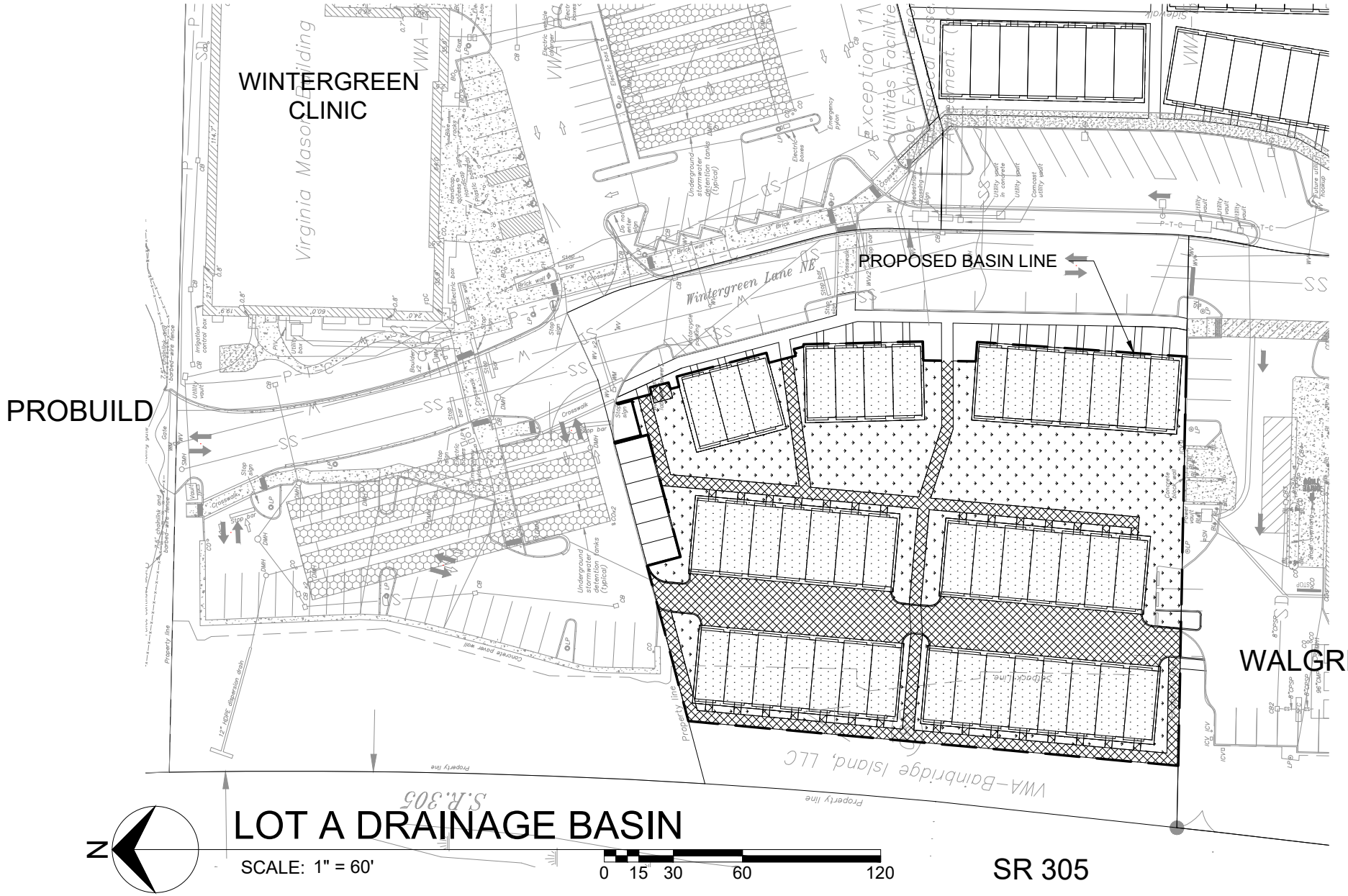

LOT A DRAINAGE BASIN

N

SCALE: 1" = 60'

0 15 30 60 120

SR 305

HARDSCAPE AREA = 7428 SF

BUILDING AREA = 18726 SF

LANDSCAPING AREA = 12027 SF

LOT 4 DRAINAGE BASIN

WINTERGREEN TOWNHOMES

PRELIMINARY PLAT SUBMITTAL

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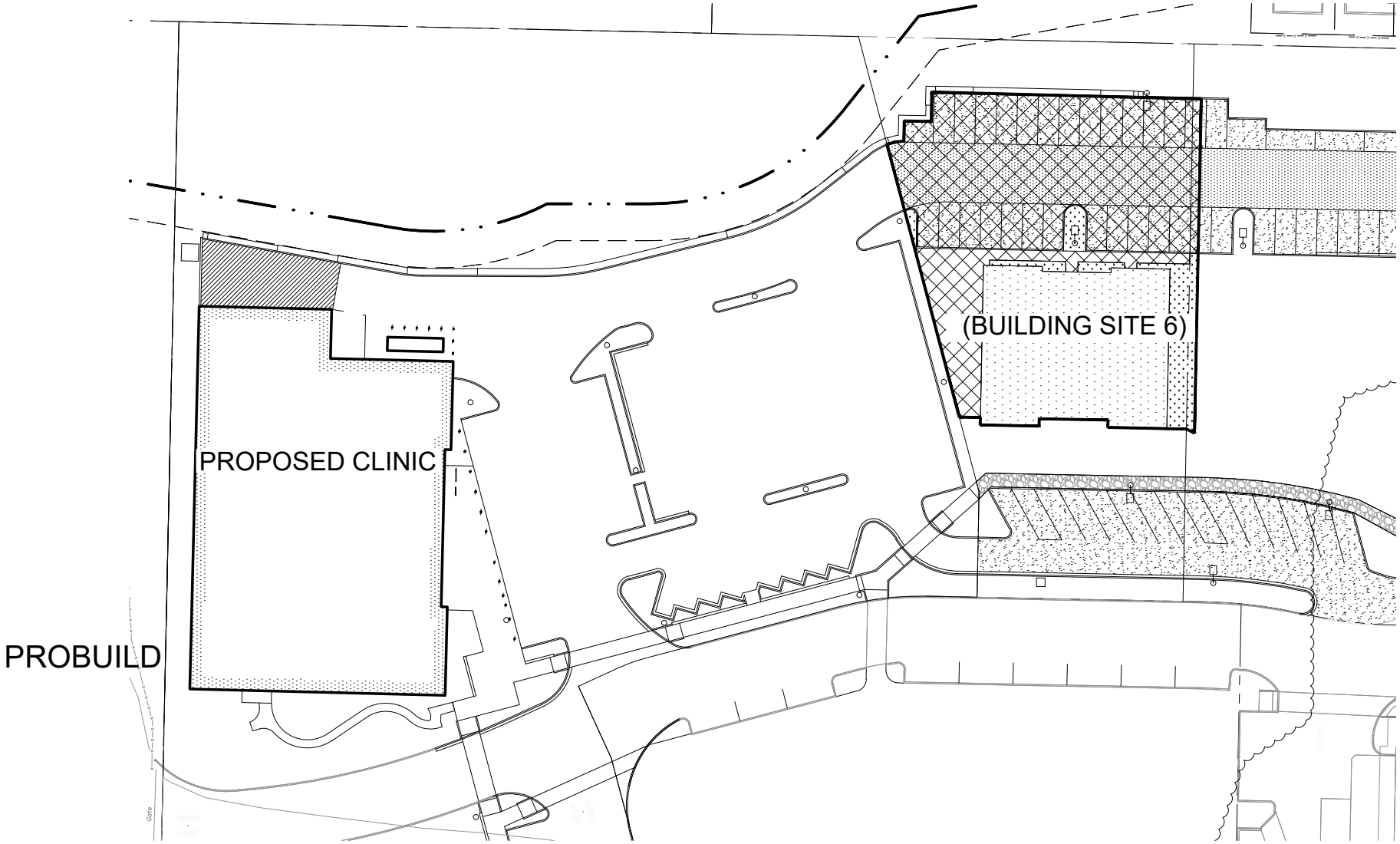

SITE 6 NORTHEAST DRAINAGE BASIN

N

SCALE: 1" = 60'

0 15 30 60 120

HARDSCAPE AREA = 9046 SF

BUILDING AREA =4800 SF

LANDSCAPING AREA = 1170 SF

SITE 6 NE DRAINAGE BASIN

LOT 6/7

PLAT UTILITY PERMIT SUBMITTAL

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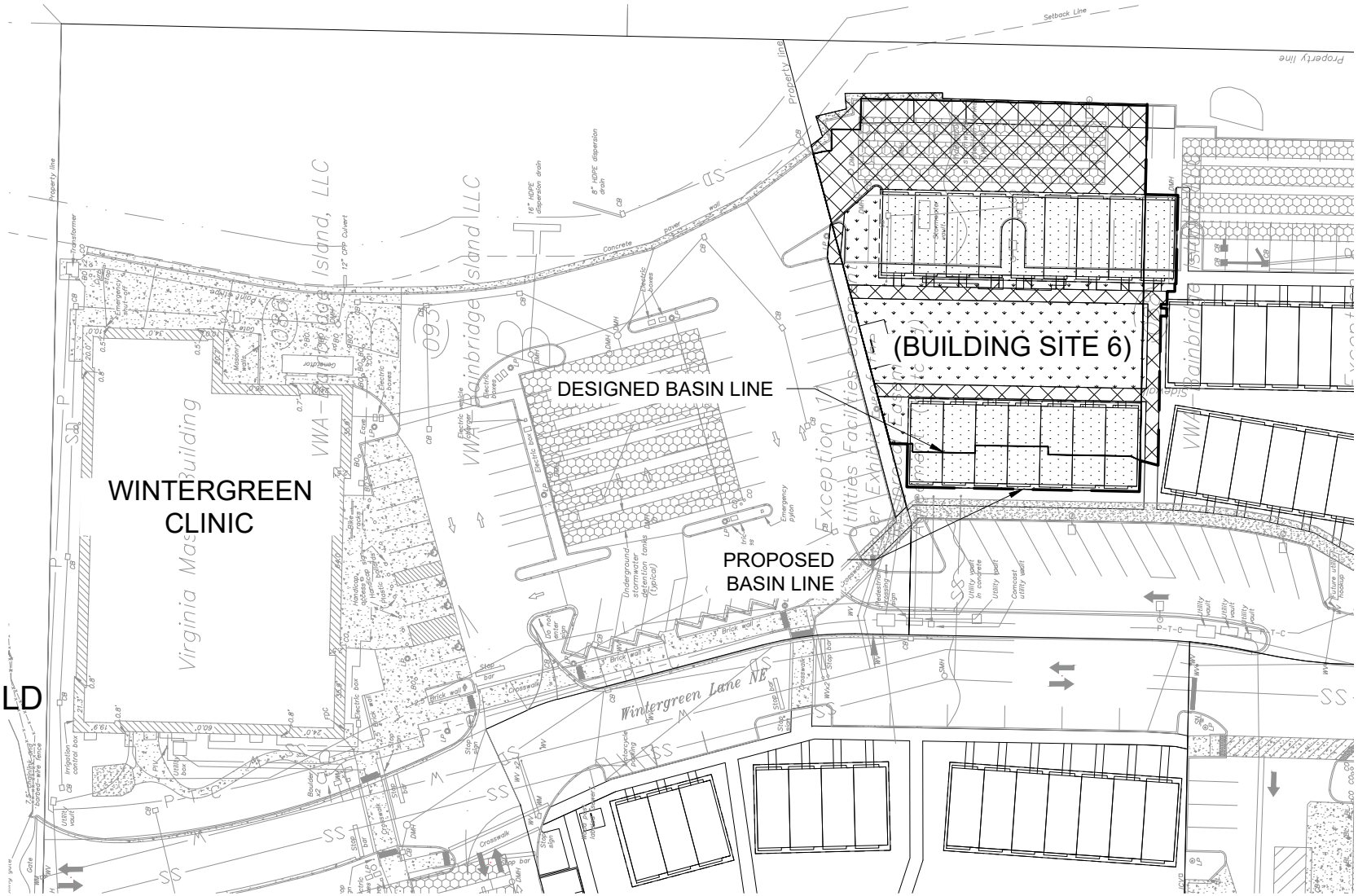

SITE 6 NORTHEAST DRAINAGE BASIN

N

SCALE: 1" = 60'

0 15 30 60 120

HARDSCAPE AREA = 6369 SF

BUILDING AREA = 6913 SF

LANDSCAPING AREA = 4292 SF

SITE 6 DRAINAGE BASIN

WINTERGREEN TOWNHOMES

PRELIMINARY PLAT SUBMITTAL

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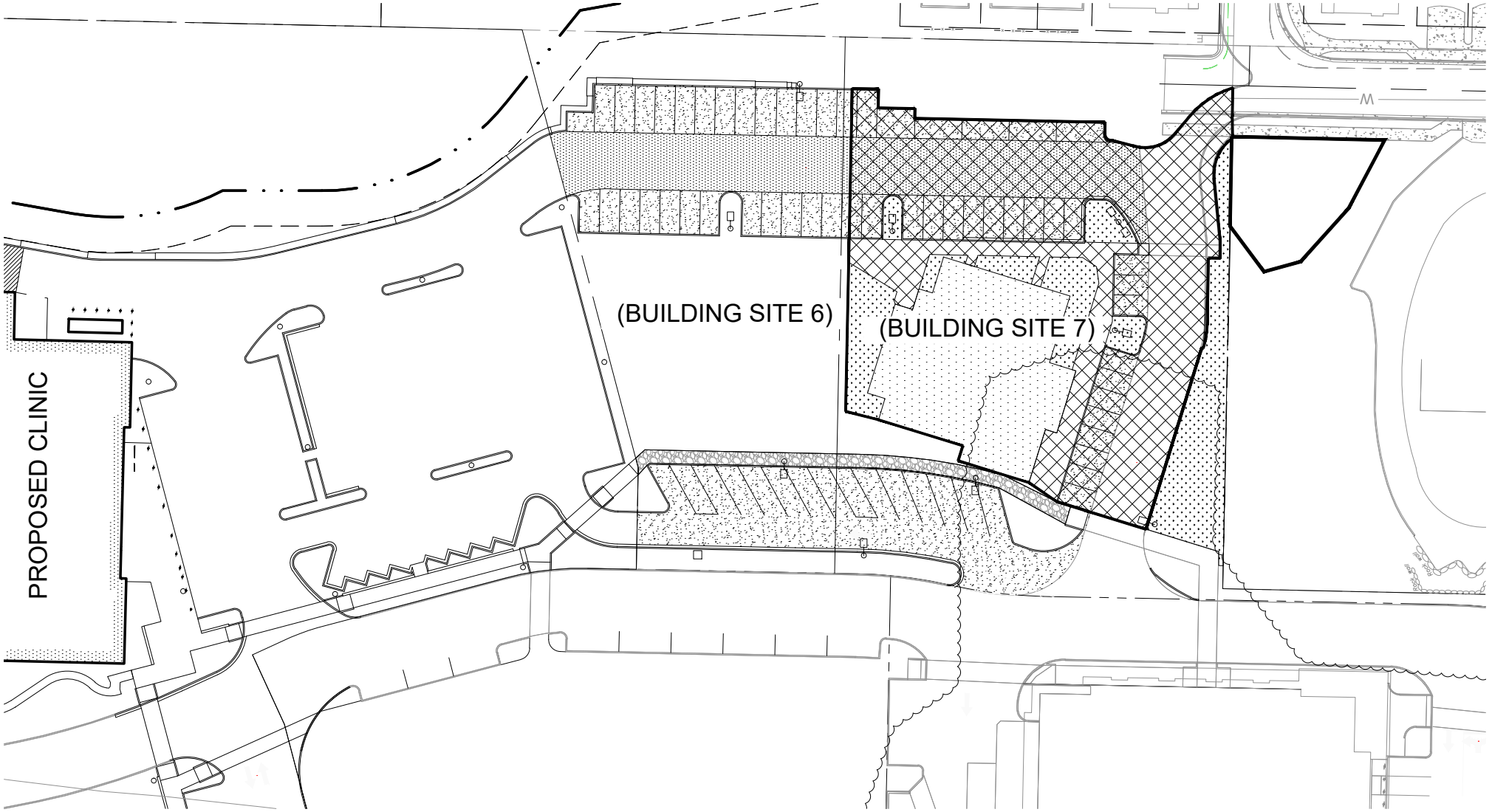

SITE 7 NORTHEAST DRAINAGE BASIN

N

SCALE: 1" = 60'

0 15 30 60 120

HARDSCAPE AREA = 14432 SF

BUILDING AREA = 5444 SF

LANDSCAPING AREA = 2561 SF

SITE 7 NE DRAINAGE BASIN

LOT 6/7

PLAT UTILITY PERMIT SUBMITTAL

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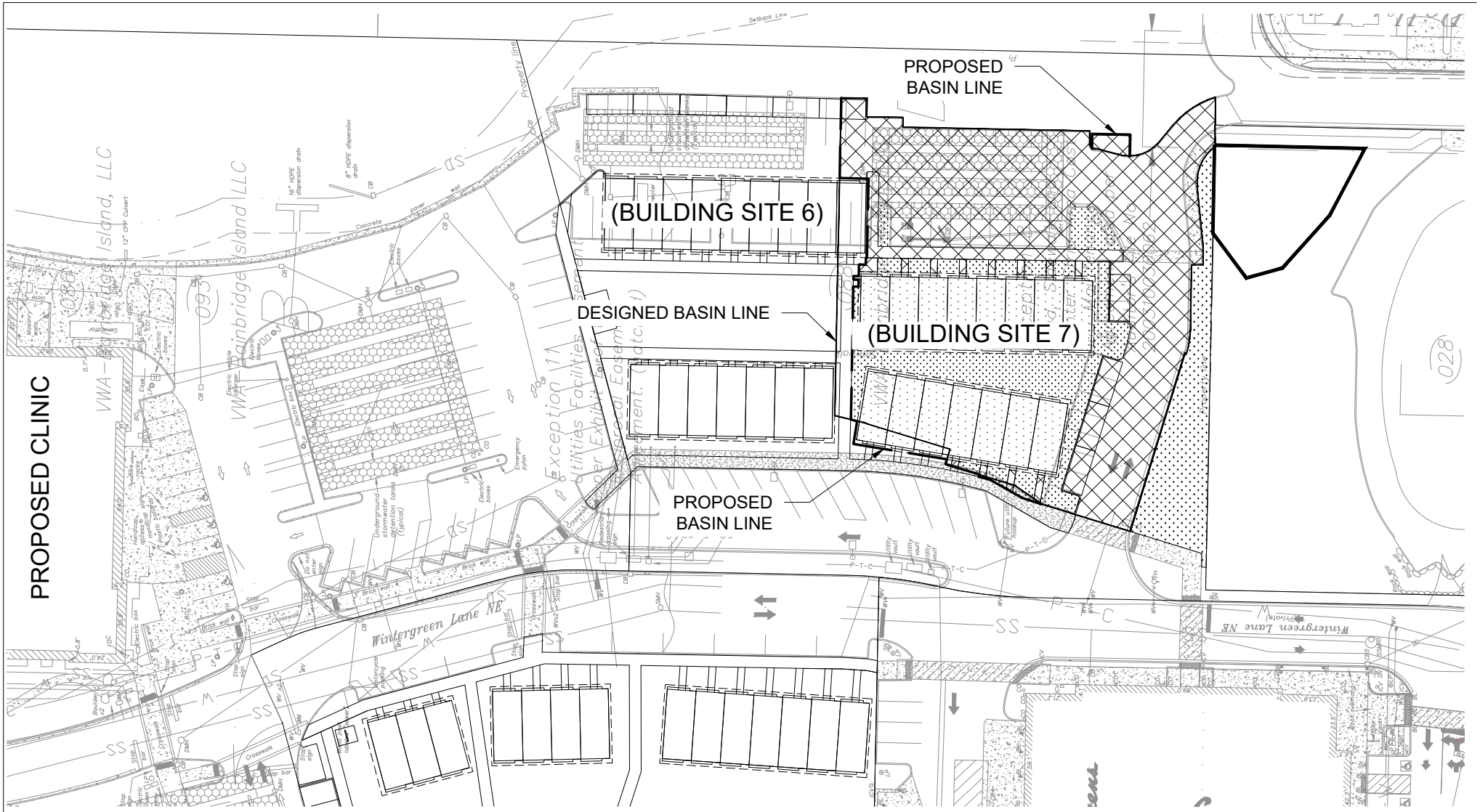

SITE 7 NORTHEAST DRAINAGE BASIN

N

SCALE: 1" = 60'

0 15 30 60 120

HARDSCAPE AREA = 12196 SF

BUILDING AREA = 6490 SF

LANDSCAPING AREA = 3329 SF

SITE 7 DRAINAGE BASIN

WINTERGREEN TOWNHOMES

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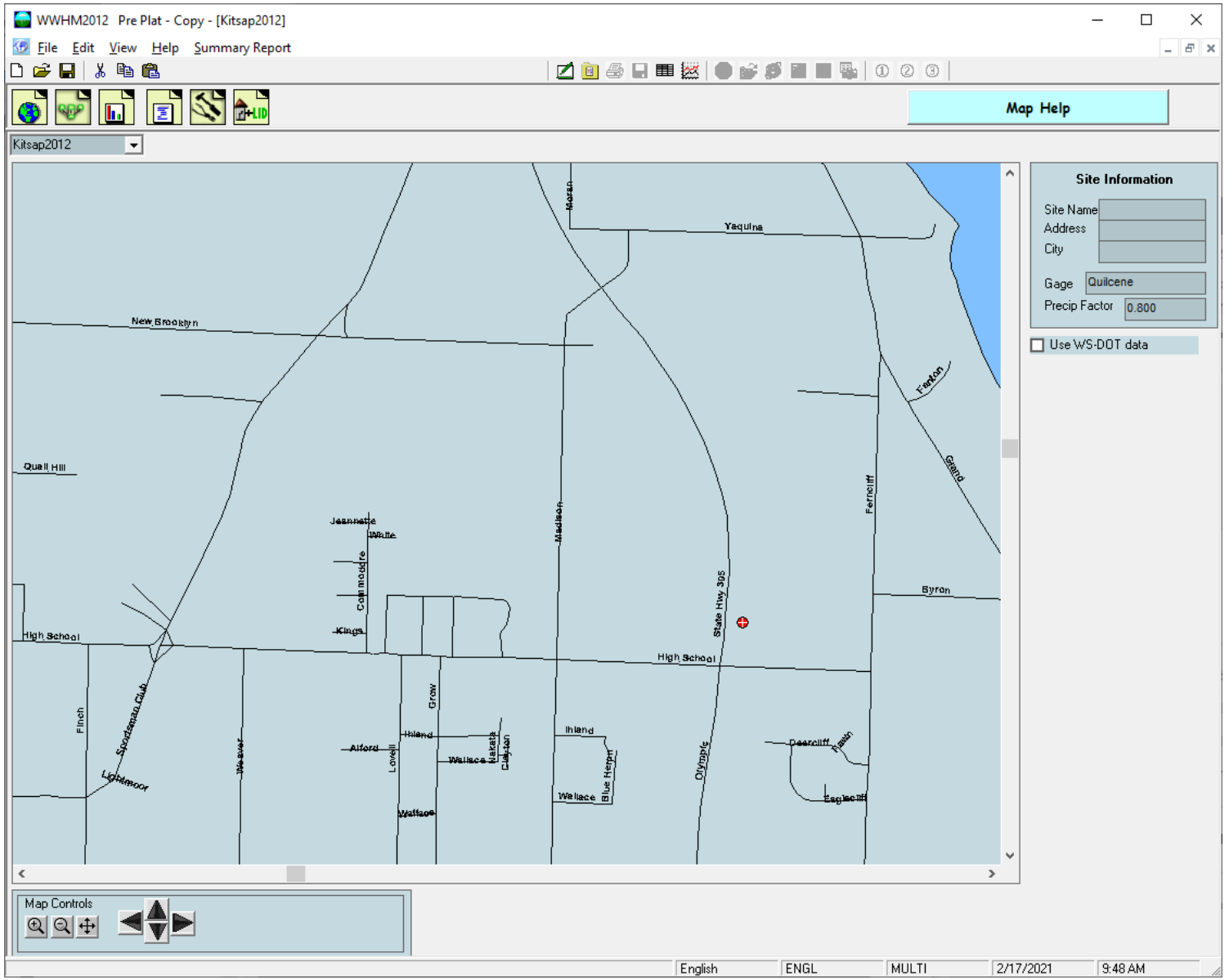
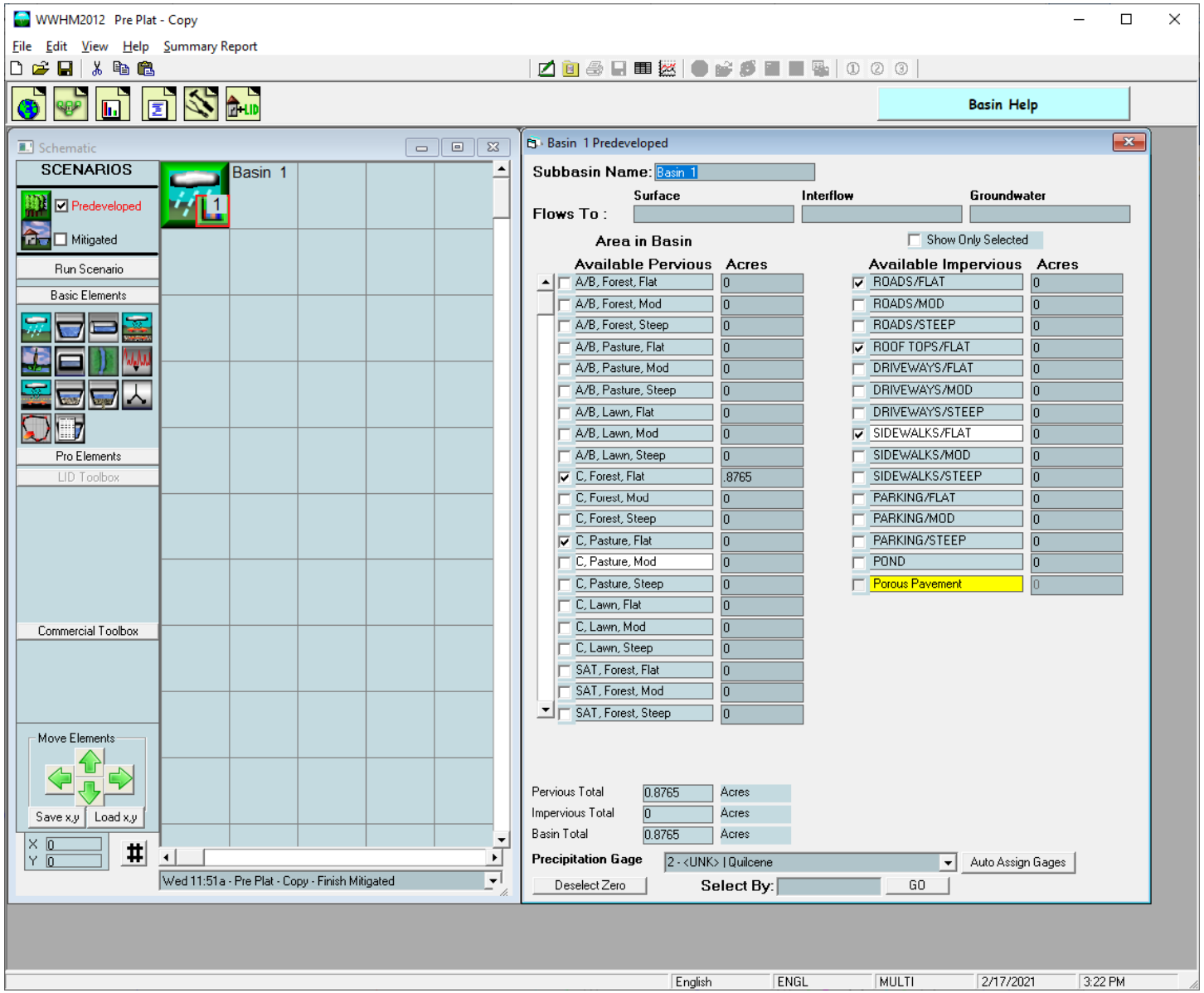
Lot D
Stormfilter Sizing

Flow/Cartridge		7.5	gpm		0.017	cfs		
		11.25	gpm		0.025	cfs		
SF1								
	Stormfilter							
	Impervious	2465	sf	0.057	ac	Qwq	0.014	cfs
	Pervious	0	sf	0.000	ac	# Cartridge	0.84 →	**1**
SF2	Stormfilter							
	Impervious	2468	sf	0.057	ac	Qwq	0.014	cfs
	Pervious	0	sf	0.000	ac	# Cartridge	0.84 →	**1**

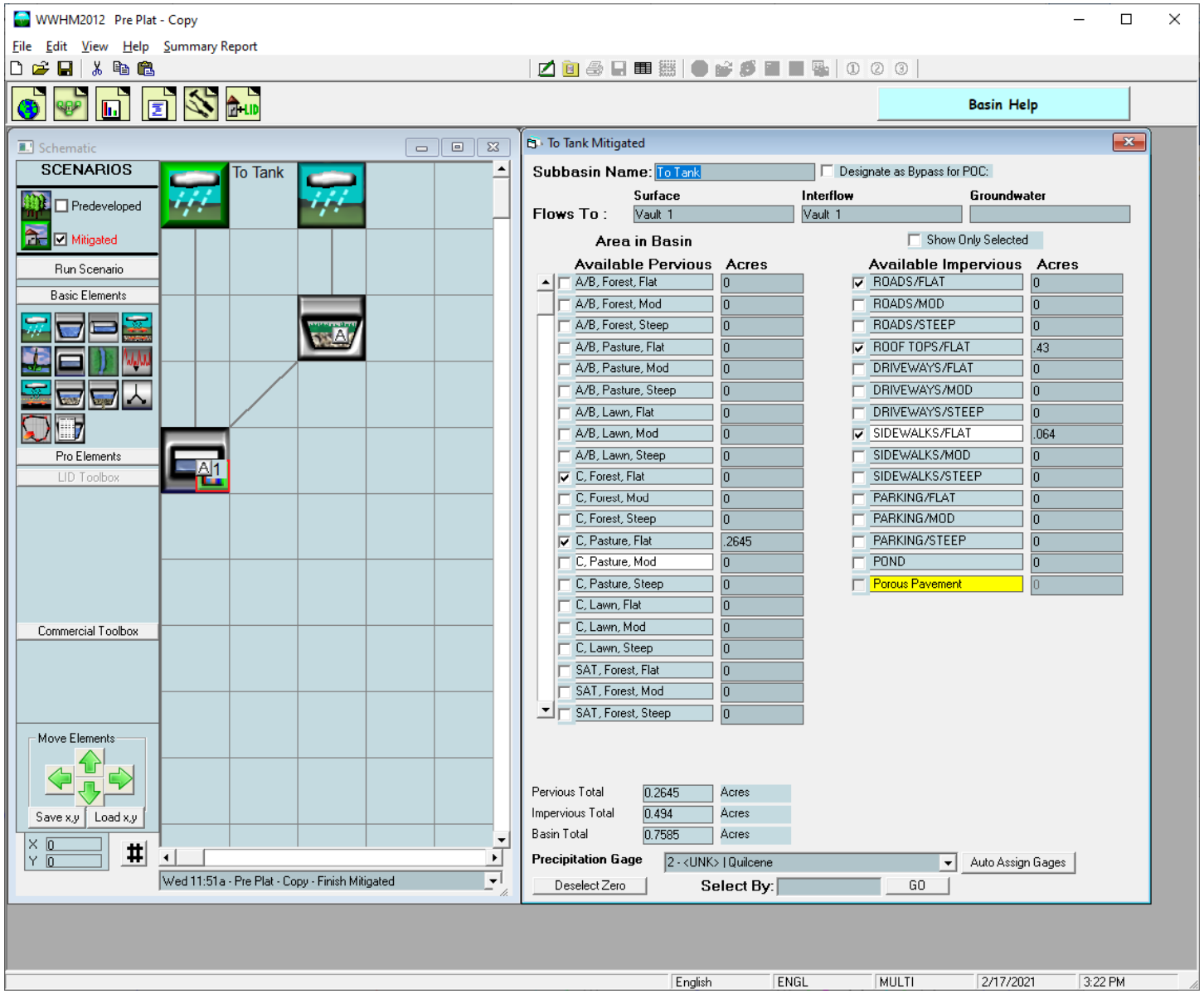
Project Location



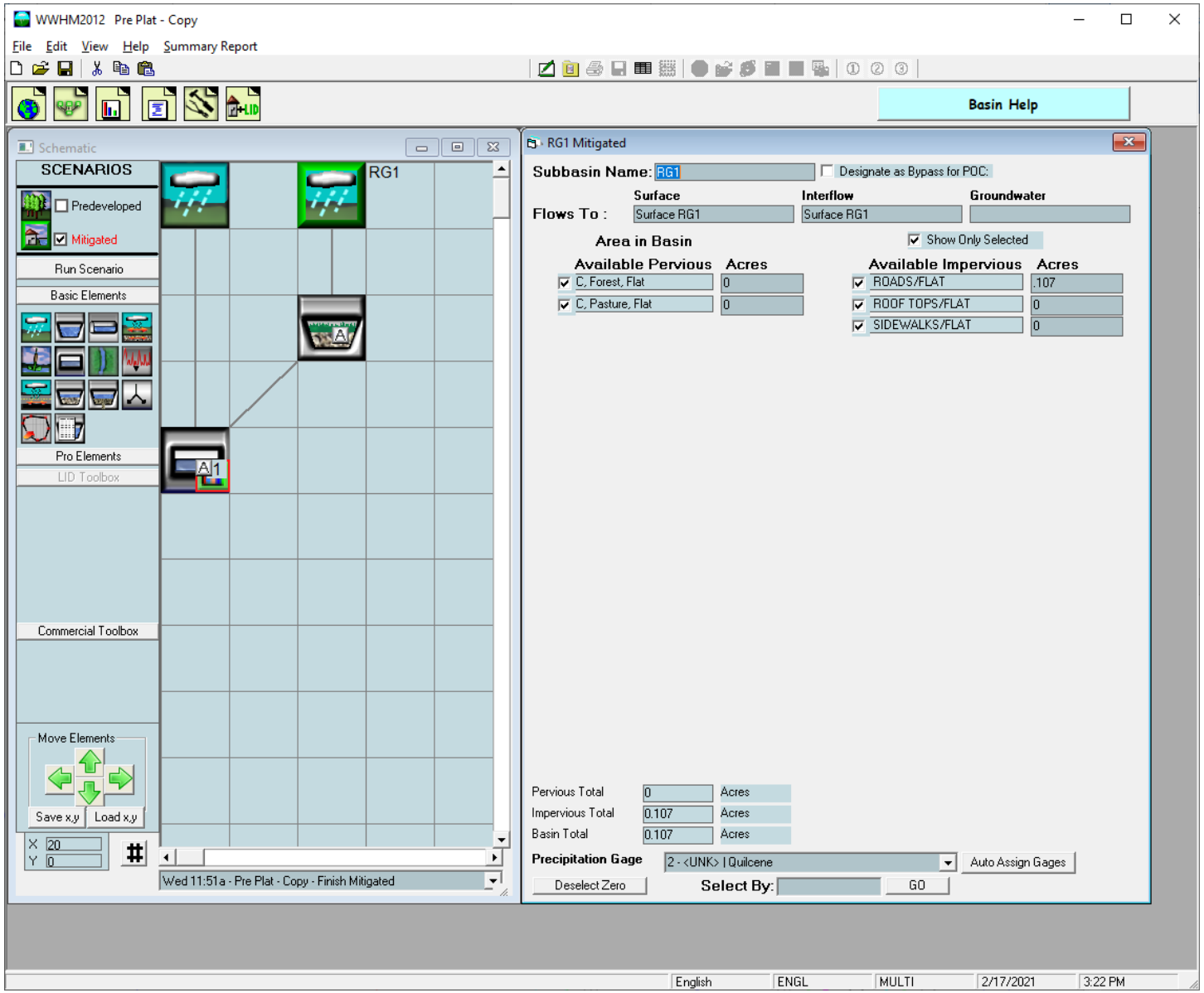
Lot A Predeveloped Basin Characteristics



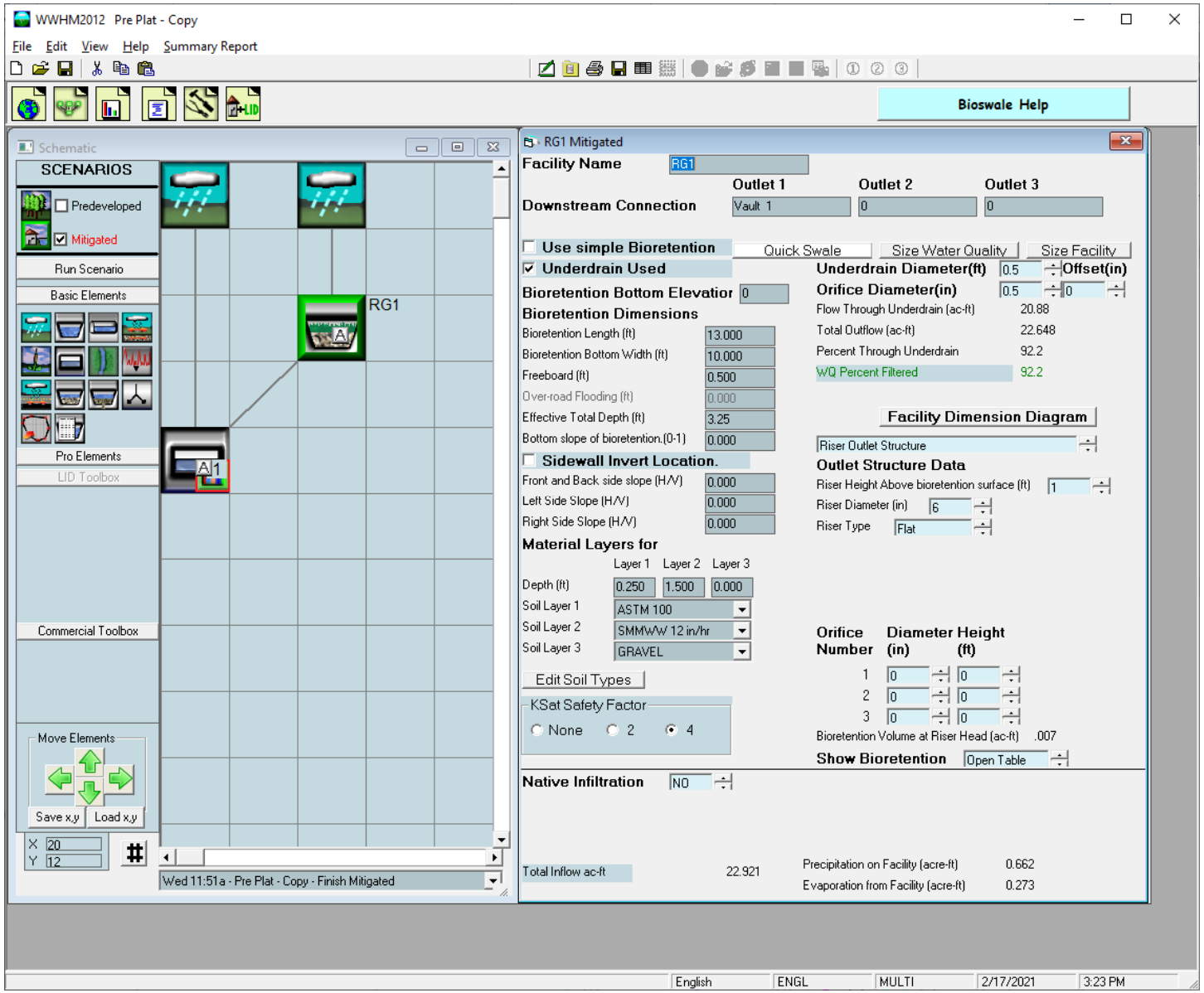
Lot A Proposed Basin Characteristics for Basin That Discharges Directly to the Proposed Detention Vault



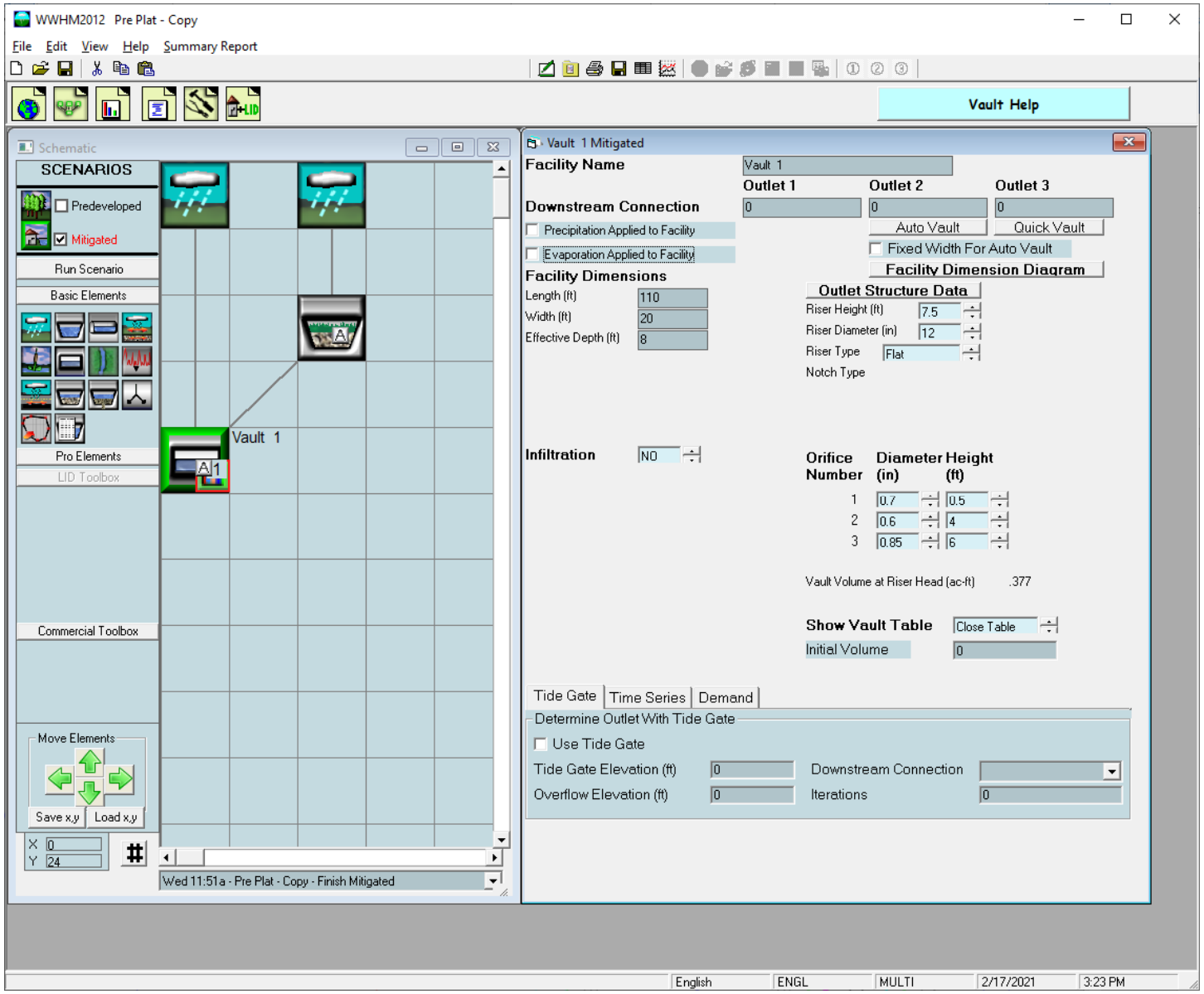
Lot A Proposed Basin Characteristics for Basin That Discharges to Rain Garden.



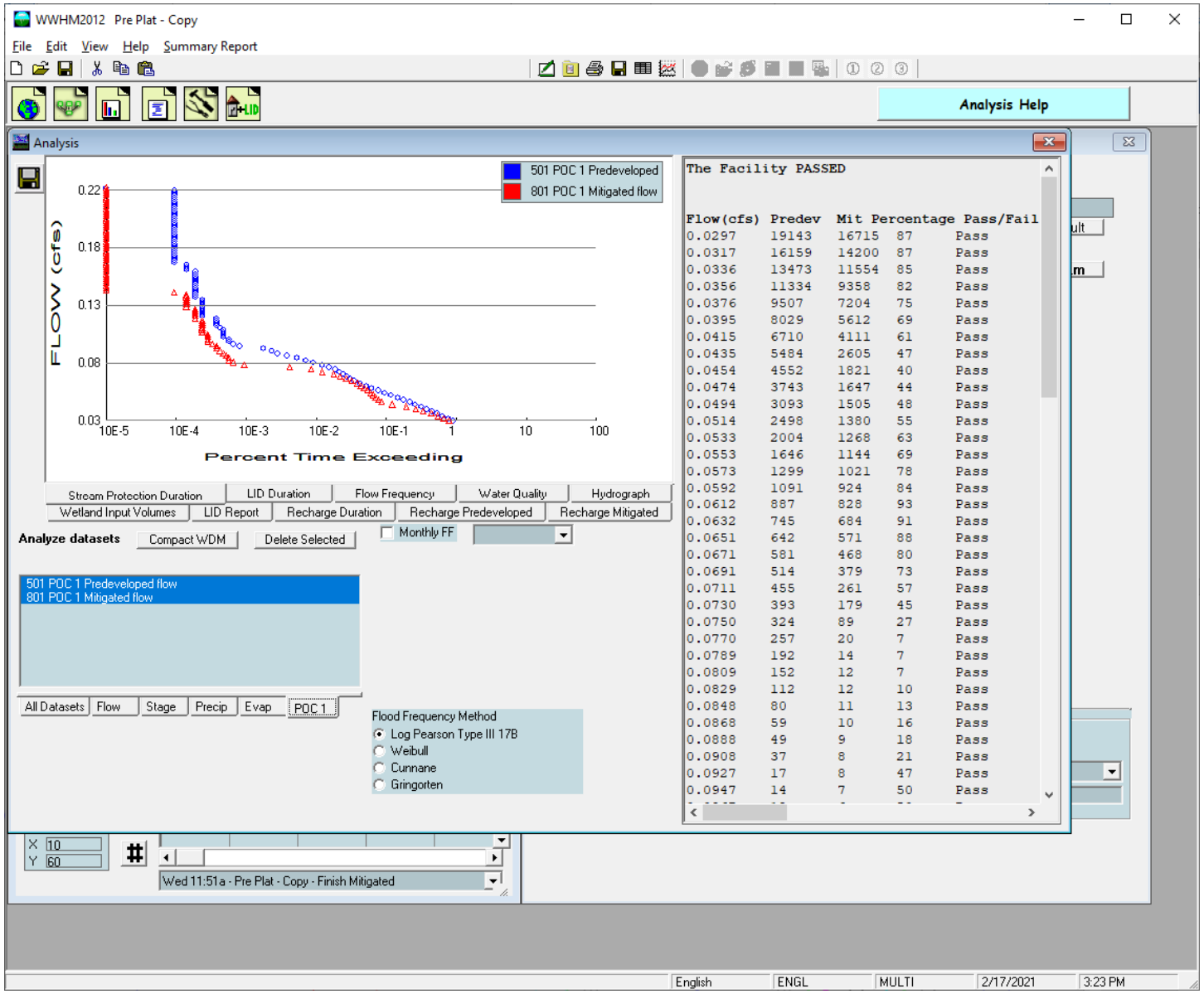
Lot A Proposed Rain Garden Dimensions and Performance



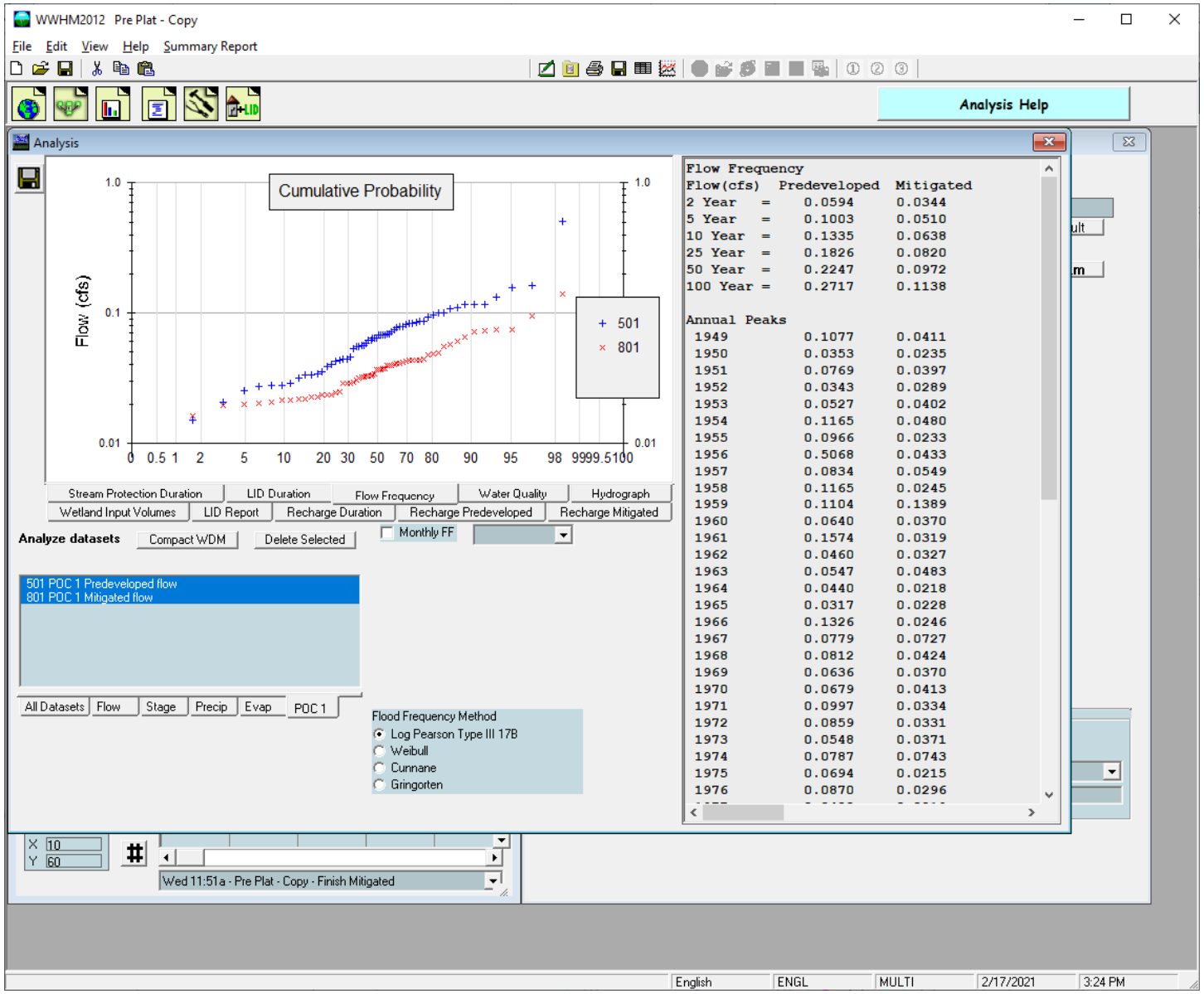
Lot A Proposed Detention Vault Dimensions



Lot A Flow Control Duration Performance



Lot A Peak Flows From Site Predeveloped Vs. Developed

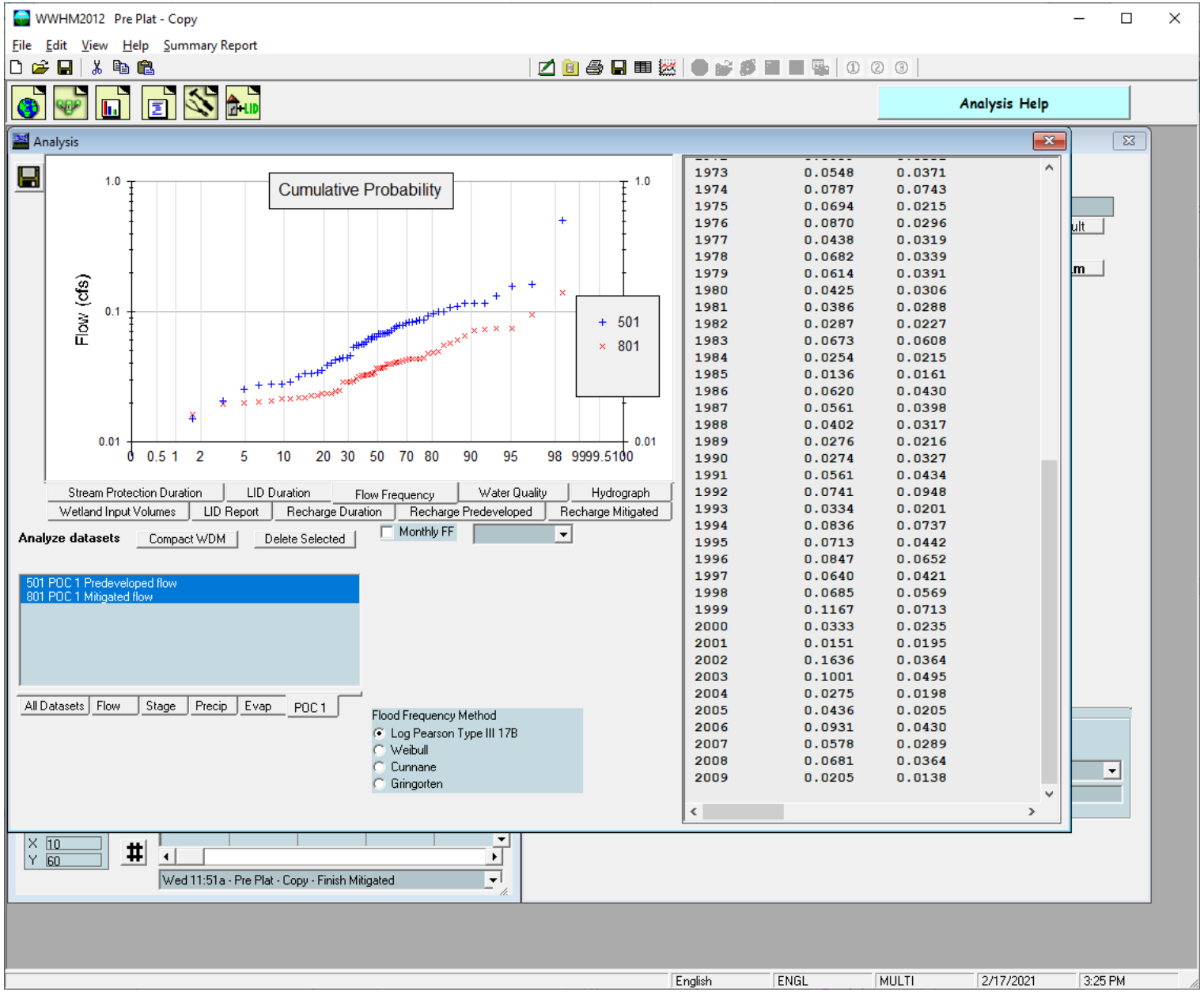


Flow Frequency

Flow(cfs)	Predeveloped	Mitigated
2 Year =	0.0594	0.0344
5 Year =	0.1003	0.0510
10 Year =	0.1335	0.0638
25 Year =	0.1826	0.0820
50 Year =	0.2247	0.0972
100 Year =	0.2717	0.1138

Annual Peaks

1949	0.1077	0.0411
1950	0.0353	0.0235
1951	0.0769	0.0397
1952	0.0343	0.0289
1953	0.0527	0.0402
1954	0.1165	0.0480
1955	0.0966	0.0233
1956	0.5068	0.0433
1957	0.0834	0.0549
1958	0.1165	0.0245
1959	0.1104	0.1389
1960	0.0640	0.0370
1961	0.1574	0.0319
1962	0.0460	0.0327
1963	0.0547	0.0483
1964	0.0440	0.0218
1965	0.0317	0.0228
1966	0.1326	0.0246
1967	0.0779	0.0727
1968	0.0812	0.0424
1969	0.0636	0.0370
1970	0.0679	0.0413
1971	0.0997	0.0334
1972	0.0859	0.0331
1973	0.0548	0.0371
1974	0.0787	0.0743
1975	0.0694	0.0215
1976	0.0870	0.0296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Flow (cfs)
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0.1
0.01
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501
801
Stream Protection Duration
LID Duration
Flow Frequency
Water Quality
Hydrograph
Wetland Input Volumes
LID Report
Recharge Duration
Recharge Predeveloped
Recharge Mitigated
Analyze datasets
Compact WDM
Delete Selected
Monthly FF
501 POC 1 Predeveloped flow
801 POC 1 Mitigated flow
All Datasets
Flow
Stage
Precip
Evap
POC 1
Flood Frequency Method
Log Pearson Type III 17B
Weibull
Cunnane
Gringorten
1973 0.0548 0.0371
1974 0.0787 0.0743
1975 0.0694 0.0215
1976 0.0870 0.0296
1977 0.0438 0.0319
1978 0.0682 0.0339
1979 0.0614 0.0391
1980 0.0425 0.0306
1981 0.0386 0.0288
1982 0.0287 0.0227
1983 0.0673 0.0608
1984 0.0254 0.0215
1985 0.0136 0.0161
1986 0.0620 0.0430
1987 0.0561 0.0398
1988 0.0402 0.0317
1989 0.0276 0.0216
1990 0.0274 0.0327
1991 0.0561 0.0434
1992 0.0741 0.0948
1993 0.0334 0.0201
1994 0.0836 0.0737
1995 0.0713 0.0442
1996 0.0847 0.0652
1997 0.0640 0.0421
1998 0.0685 0.0569
1999 0.1167 0.0713
2000 0.0333 0.0235
2001 0.0151 0.0195
2002 0.1636 0.0364
2003 0.1001 0.0495
2004 0.0275 0.0198
2005 0.0436 0.0205
2006 0.0931 0.0430
2007 0.0578 0.0289
2008 0.0681 0.0364
2009 0.0205 0.0138
X 10
Y 60
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ENGL
MULTI
2/17/2021
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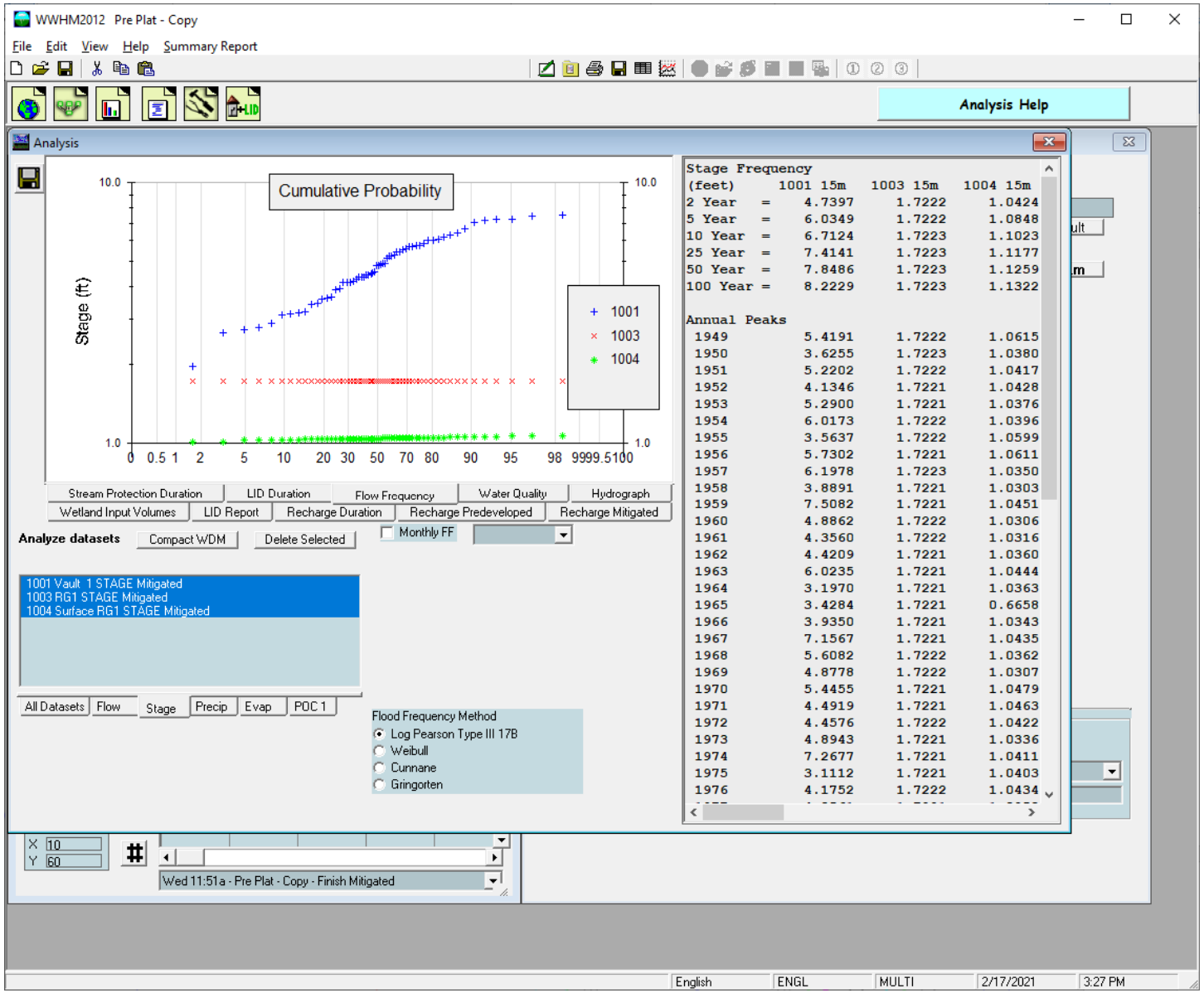
Lot A Maximum Stage in Detention Vault and Rain Garden

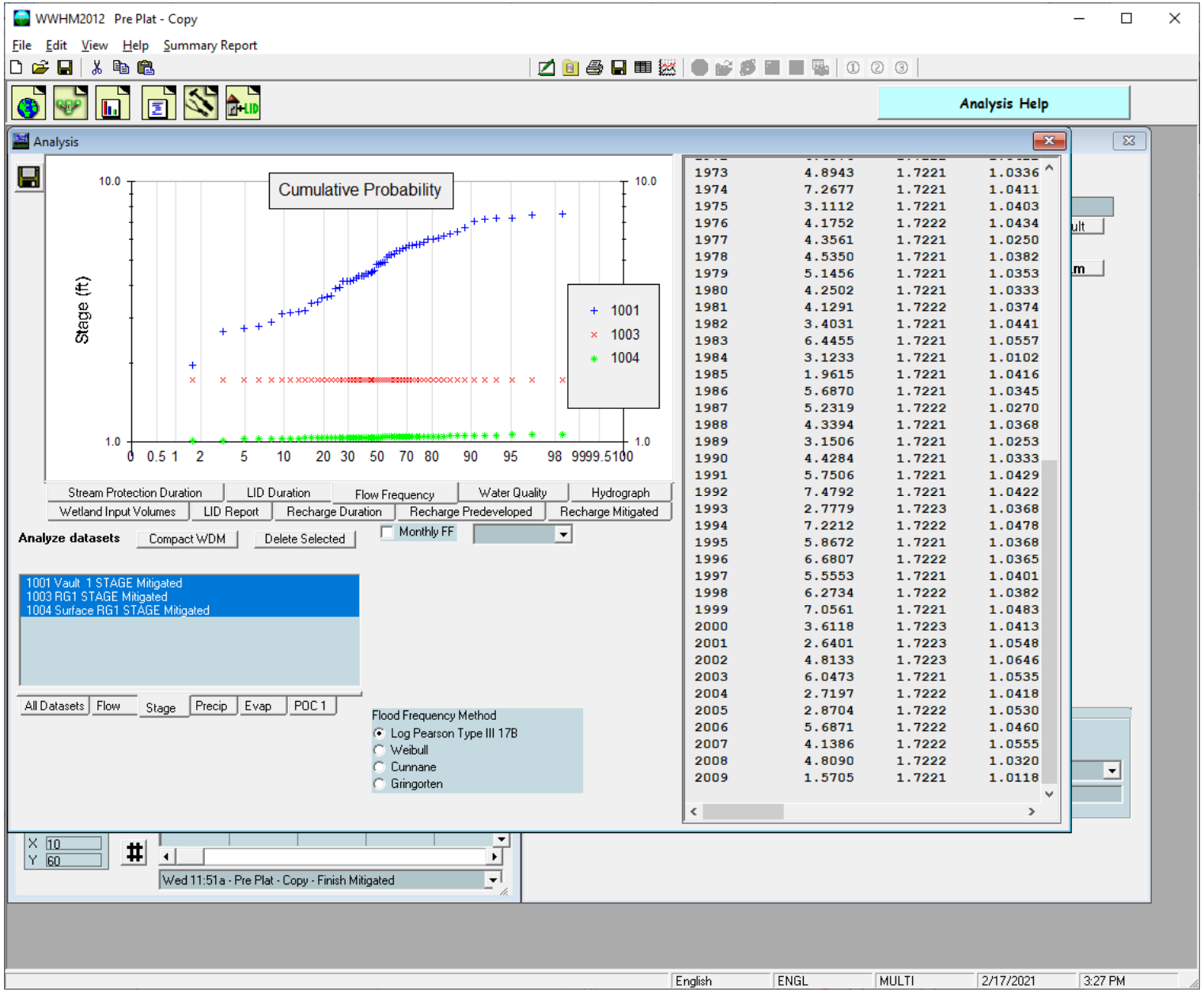


Stage Frequency

(feet)	1001 15m	1003 15m	1004 15m
2 Year =	4.7397	1.7222	1.0424
5 Year =	6.0349	1.7222	1.0848
10 Year =	6.7124	1.7223	1.1023
25 Year =	7.4141	1.7223	1.1177
50 Year =	7.8486	1.7223	1.1259
100 Year =	8.2229	1.7223	1.1322

Annual Peaks

1949	5.4191	1.7222	1.0615
1950	3.6255	1.7223	1.0380
1951	5.2202	1.7222	1.0417
1952	4.1346	1.7221	1.0428
1953	5.2900	1.7221	1.0376
1954	6.0173	1.7222	1.0396
1955	3.5637	1.7222	1.0599
1956	5.7302	1.7221	1.0611
1957	6.1978	1.7223	1.0350
1958	3.8891	1.7221	1.0303
1959	7.5082	1.7221	1.0451
1960	4.8862	1.7222	1.0306
1961	4.3560	1.7222	1.0316
1962	4.4209	1.7221	1.0360
1963	6.0235	1.7221	1.0444
1964	3.1970	1.7221	1.0363
1965	3.4284	1.7221	0.6658
1966	3.9350	1.7221	1.0343
1967	7.1567	1.7221	1.0435
1968	5.6082	1.7222	1.0362
1969	4.8778	1.7222	1.0307
1970	5.4455	1.7221	1.0479
1971	4.4919	1.7221	1.0463
1972	4.4576	1.7222	1.0422
1973	4.8943	1.7221	1.0336
1974	7.2677	1.7221	1.0411
1975	3.1112	1.7221	1.0403
1976	4.1752	1.7222	1.0434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Analysis Help
Analysis
Cumulative Probability
Stage (ft)
10.0
1.0
0 0.5 1 2 5 10 20 30 50 70 80 90 95 98 99 99.5 100
1001
1003
1004
Stream Protection Duration
LID Duration
Flow Frequency
Water Quality
Hydrograph
Wetland Input Volumes
LID Report
Recharge Duration
Recharge Predeveloped
Recharge Mitigated
Analyze datasets
Compact WDM
Delete Selected
Monthly FF
1001 Vault 1 STAGE Mitigated
1003 RG1 STAGE Mitigated
1004 Surface RG1 STAGE Mitigated
All Datasets
Flow
Stage
Precip
Evap
POC 1
Flood Frequency Method
Log Pearson Type III 17B
Weibull
Cunnane
Gringorten
1973 4.8943 1.7221 1.0336
1974 7.2677 1.7221 1.0411
1975 3.1112 1.7221 1.0403
1976 4.1752 1.7222 1.0434
1977 4.3561 1.7221 1.0250
1978 4.5350 1.7221 1.0382
1979 5.1456 1.7221 1.0353
1980 4.2502 1.7221 1.0333
1981 4.1291 1.7222 1.0374
1982 3.4031 1.7221 1.0441
1983 6.4455 1.7221 1.0557
1984 3.1233 1.7221 1.0102
1985 1.9615 1.7221 1.0416
1986 5.6870 1.7221 1.0345
1987 5.2319 1.7222 1.0270
1988 4.3394 1.7221 1.0368
1989 3.1506 1.7221 1.0253
1990 4.4284 1.7221 1.0333
1991 5.7506 1.7221 1.0429
1992 7.4792 1.7221 1.0422
1993 2.7779 1.7223 1.0368
1994 7.2212 1.7222 1.0478
1995 5.8672 1.7221 1.0368
1996 6.6807 1.7222 1.0365
1997 5.5553 1.7221 1.0401
1998 6.2734 1.7222 1.0382
1999 7.0561 1.7221 1.0483
2000 3.6118 1.7223 1.0413
2001 2.6401 1.7223 1.0548
2002 4.8133 1.7223 1.0646
2003 6.0473 1.7221 1.0535
2004 2.7197 1.7222 1.0418
2005 2.8704 1.7222 1.0530
2006 5.6871 1.7222 1.0460
2007 4.1386 1.7222 1.0555
2008 4.8090 1.7222 1.0320
2009 1.5705 1.7221 1.0118
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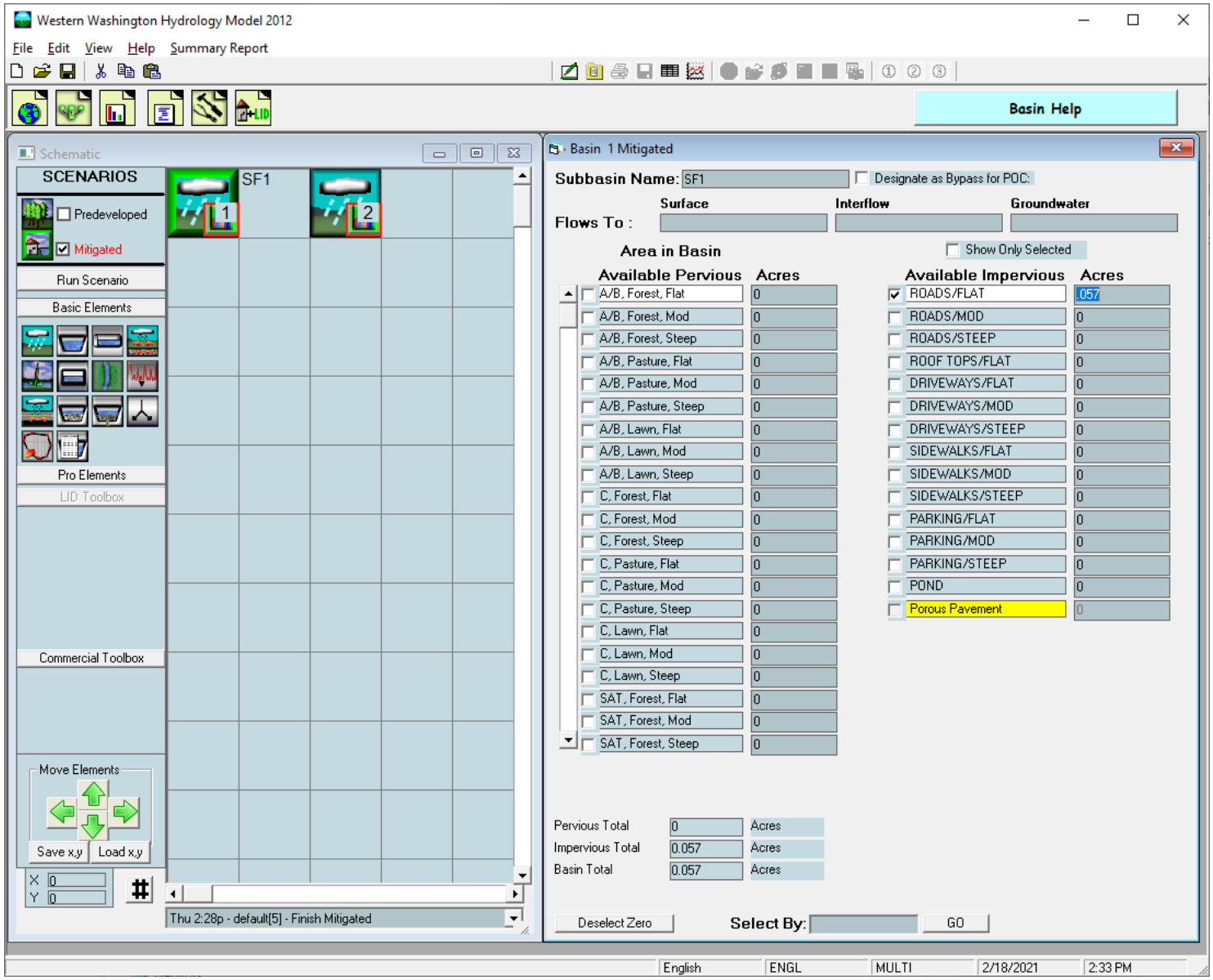
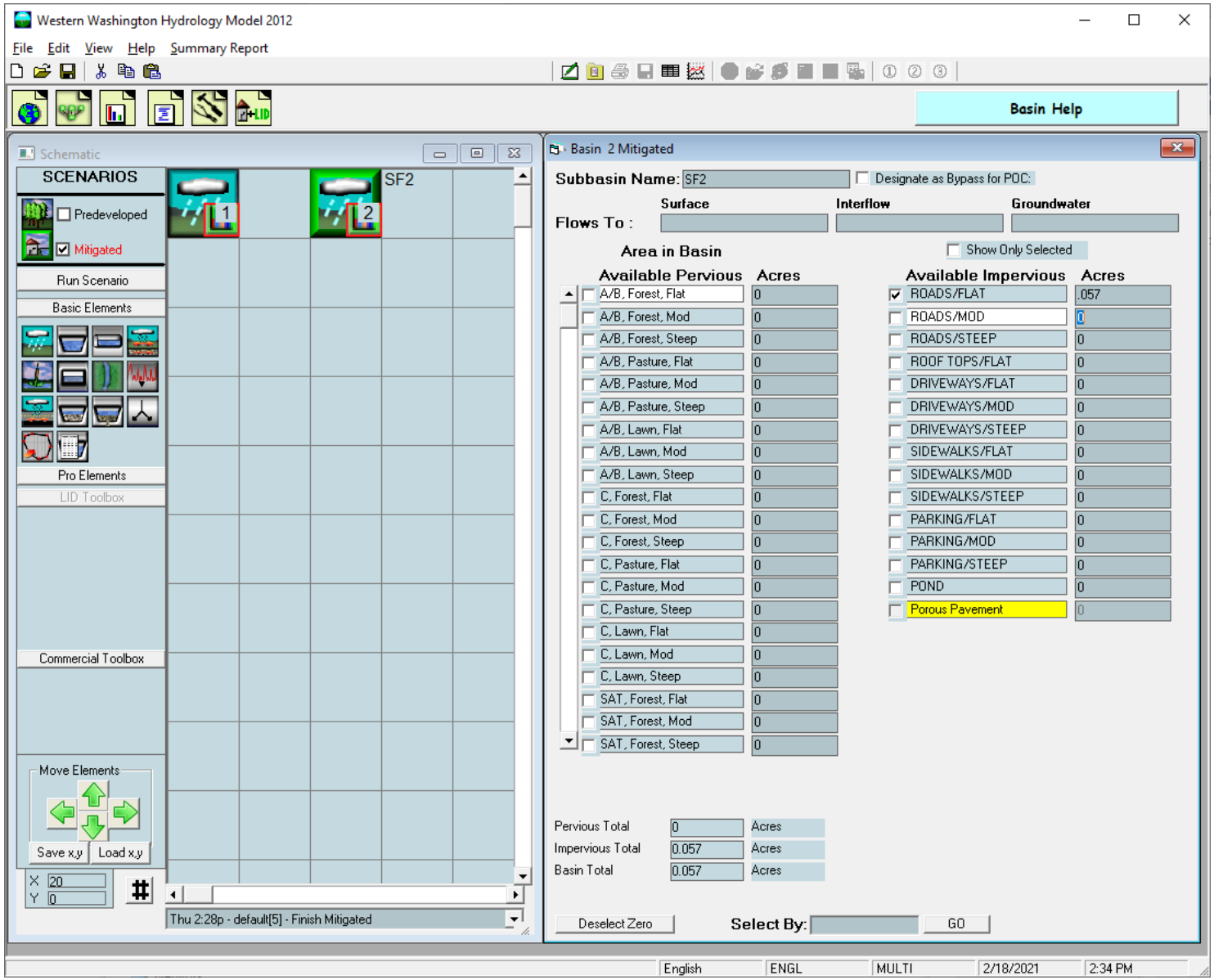
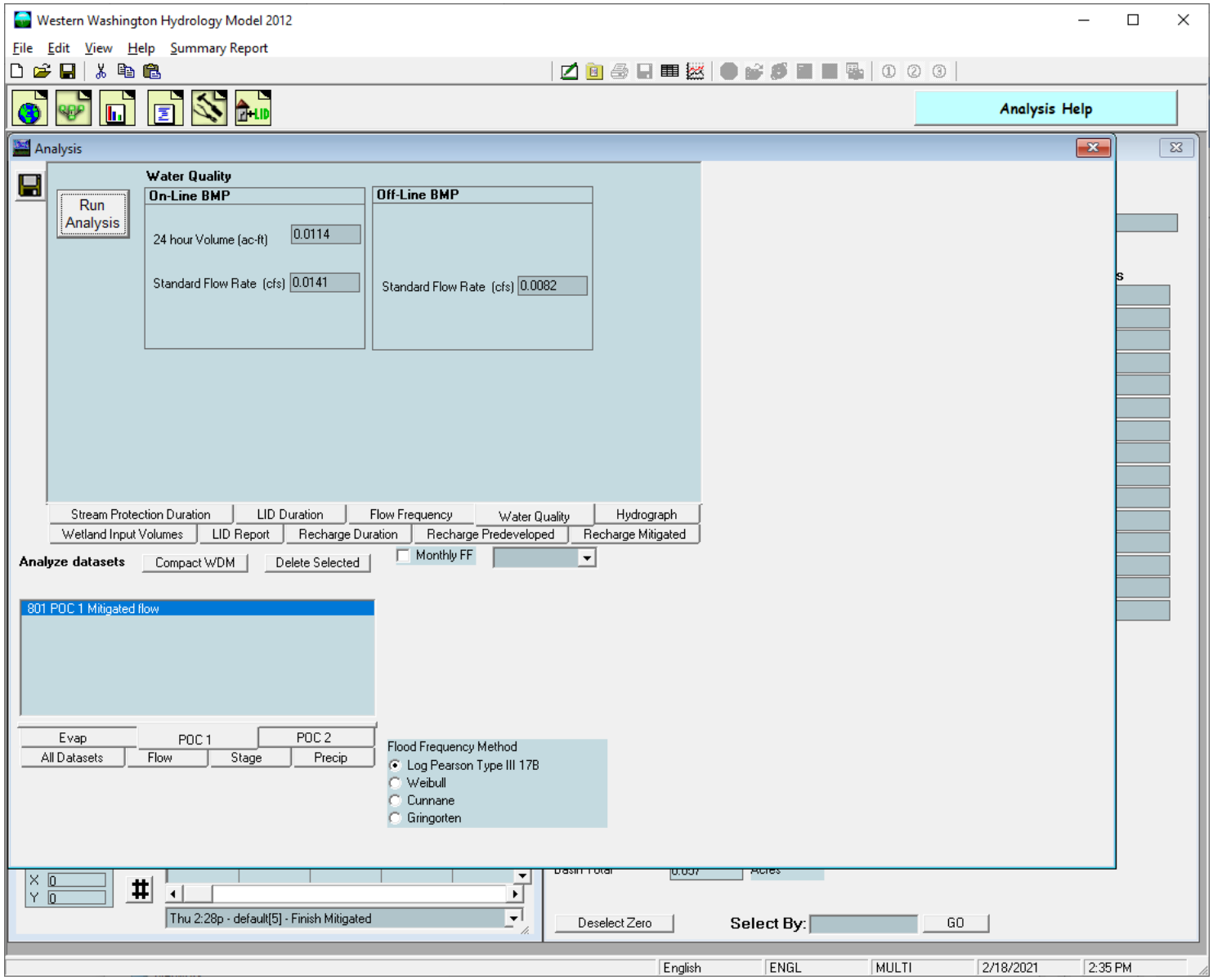
Lot D Stormfilter 1 Basin Characteristics



Lot D Stormfilter 2 Basin Characteristics



Lot D Stormfilter 1 Water Quality Flow Rate



Lot D Stormfilter 2 Water Quality Flow Rate